

PROJECT CONTACTS

OWNER

DNL HOLDINGS, LLC 2000 WEST STADIUM BLVD. ANN ARBOR, MI 48107

ARCHITECT

METRO GROUP ARCHITECTS P.O. BOX 7363 ANN ARBOR, MI 48107 PHONE: 734.747.8999

CONTRACTOR / DEVELOPER

A. R. BROUWER COMPANY. CONTACT: DAVID NISWONGER DEXTER, MI 48130 PHONE: 734.426.9980

ENGINEER / SURVEYOR

METRO CONSULTING ASSOCIATES CONTACT: CHRISTOPHER SUTTON 45345 FIVE MILE ROAD PLYMOUTH, MI 48170 PHONE: 800.525.6016 EMAIL: CSUTTON@METROCA.NET

MUNICIPALITY

CITY OF ANN ARBOR 301 EAST HURON STREET ANN ARBOR, MI 48104 PHONE: 734.994.2700 OR 734.794.6320

PROJECT NARRATIVE

LEWIS JEWELERS IS RESPECTFULLY REQUESTING SITE PLAN APPROVAL TO ALLOW FOR REDEVELOPMENT OF THE EXISTING SITE AT 300 S MAPLE ROAD TO BE USED FOR NEW RETAIL BUILDINGS.

THE OVERALL EXISTING PARCEL IS 1.98 ACRES IN SIZE AND IS ZONED C3: FRINGE COMMERCIAL DISTRICT. THE SITE IS CURRENTLY VACANT AND WAS MOST RECENTLY USED AS THE QUARTER BISTRO RESTAURANT. THE EXISTING BUILDING AND SITE IMPROVEMENTS WILL BE DEMOLISHED TO ACCOMMODATE THREE NEW COMMERCIAL BUILDINGS TOTALING 24,529 SF OF FLOOR AREA. SITE WORK WILL INCLUDE NEW PARKING SPACES AND DRIVE AISLES, A SIDEWALK CONNECTION FROM MAPLE ROAD TO THE WESTGATE SHOPPING CENTER, BIORETENTION AREAS, AN UNDERGROUND DETENTION SYSTEM, AND LANDSCAPING.

WATER SERVICE WILL BE PROVIDED TO THE BUILDINGS VIA A NEW 4" FIRE SERVICE AND 2" DOMESTIC SERVICE CONNECTED TO THE EXISTING 12" WATER MAIN ON THE SOUTH SIDE OF THE SITE. SANITARY SEWER SERVICE WILL BE PROVIDED FROM THE EXISTING 6" SANITARY SEWER ON THE NORTH SIDE OF THE SITE. STORMWATER MANAGEMENT WILL BE HANDLED VIA TWO UNDERGROUND DETENTION SYSTEMS ON THE NORTH AND SOUTH SIDES OF THE SITE. ADDITIONALLY, THERE WILL BE SIX BIORETENTION AREAS THROUGHOUT THE SITE. THE WIDTH OF THE EXISTING CURB CUT ONTO S MAPLE ROAD WILL BE REDUCED TO ACCOMMODATE THE PROPOSED BUILDINGS WHILE STILL PROVIDING TWO LANES OF TRAFFIC. THREE ACCESS POINTS TO THE EXISTING DRIVEWAYS FOR THE WESTGATE SHOPPING CENTER WILL ALSO BE MAINTAINED.

CITY OF ANN ARBOR GENERAL NOTES

- 1) THE CONSTRUCTION COVERED BY THESE PLANS SHALL CONFORM TO THE CITY OF ANN ARBOR PUBLIC SERVICES STANDARD SPECIFICATIONS WHICH ARE INCLUDED BY REFERENCE.
- 2) THE OMISSION OF ANY STANDARD DETAIL DOES NOT RELIEVE THE CONTRACTORS OF THEIR OBLIGATION TO CONSTRUCT ITEMS IN COMPLETE ACCORDANCE WITH THE PUBLIC SERVICES STANDARD SPECIFICATIONS.
- 3) USE OF LINE STOPS IS REQUIRED WHERE EXISTING WATER MAINS CANNOT BE SUFFICIENTLY ISOLATED TO COMPLETE THE WORK. THE COST OF ANY LINE STOP ISOLATION IS THE RESPONSIBILITY OF THE DEVELOPER AND/OR CONTRACTOR.
- 4) PAVEMENT MARKINGS DISTURBED DUE TO PAVEMENT CUTS OR CONSTRUCTION RELATED ACTIVITIES SHALL BE REPLACED AS DIRECTED BY ENGINEER. REPLACEMENT DURING CONSTRUCTION OF THE PROJECT MAY BE CONSIDERED TEMPORARY, WITH FINAL PAVEMENT MARKING RESTORATION TO OCCUR AT THE END OF THE PROJECT.

OPEN SPACE SUMMARY

EXISTING OPEN SPACE AREA: 18.960 SF (0.435 AC) PROPOSED OPEN SPACE AREA: 12,263 SF (0.282 AC)

SHEET INDEX

COVER SHEET

ALTA SURVEY

DEMOLITION PLAN

LAYOUT PLAN

GRADING & SOIL EROSION CONTROL PLAN

SOIL EROSION CONTOL NOTES AND DETAILS UTILITY PLAN

STORM WATER MANAGEMENT PLAN

STORM WATER MANAGEMENT CALCULATIONS - NORTH

STORM WATER MANAGEMENT CALCULATIONS - SOUTH

STORM WATER MANAGEMENT DETAILS

TYPICAL DETAILS

TYPICAL DETAILS

LANDSCAPE PLAN

LANDSCAPE NOTES, CALCULATIONS & NATURAL FEATURES

LANDSCAPE DETAILS

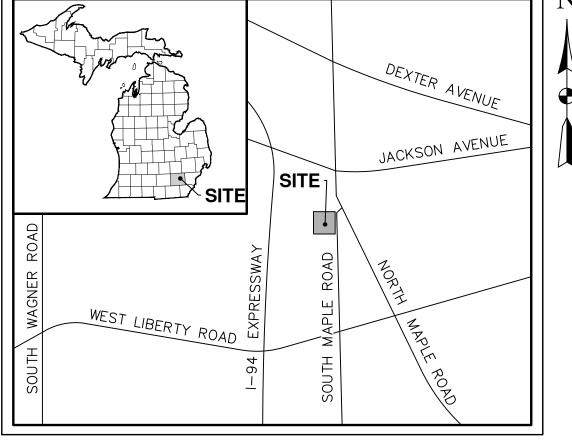
OVERALL FLOOR PLAN

OVERALL EXTERIOR ELEVATIONS

A7.0 OVERALL BUILDING SECTIONS

SITE AND BUILDING RENDERINGS

PH-1 PHOTOMETRIC PLAN



VICINITY MAP

SITE ADDRESS: 300 SOUTH MAPLE ROAD / TAX ID#: 09-08-25-103-006

LAND **DEVELOPMENT SUMMARY**

DEVELOPMENT PROGRAM

- THE PROPOSED DEVELOPMENT INCLUDES DEMOLITION OF THE EXISTING BUILDING AND CONSTRUCTION OF THREE NEW COMMERCIAL BUILDINGS. • PROPOSED BUILDING #1: +/-9,408 SF JEWELRY STORE (LEWIS
- •• PROPOSED BUILDING #2: +/-10,575 SF RETAIL BUILDING (4 UNITS) •• PROPOSED BUILDING #3: +/-4,546 SF RETAIL/RESTAURANT
- BUILDING WITH DRIVE-THRU (2 UNITS) ALL EXISTING PAVEMENT WILL BE REMOVED AND REPLACED, AND THE PROPOSED VEHICULAR USE AREA WILL BE APPROXIMATELY 31,987 SF
- INCLUDING THE PARKING SPACES AND DRIVE AISLES. • PROPOSED SIDEWALKS AND RAMPS WILL MEET CURRENT ADA
- THE SITE LANDSCAPING WILL BE BROUGHT UP TO CURRENT CITY REQUIREMENTS AND STANDARDS. • ALL IMPROVEMENTS ARE PROPOSED TO BE COMPLETED IN ONE PHASE.

- COMMUNITY ANALYSIS • THE PROPOSED PROJECT WILL HAVE NO IMPACT ON PUBLIC SCHOOLS. • THE PROPOSED PROJECT IS PART OF AN EXISTING RETAIL CENTER AND
- IS COMPATIBLE WITH THE SURROUNDING USES. • ADJACENT LAND USES WILL NOT HAVE A SIGNIFICANT IMPACT ON THE
- PROPOSED PROJECT. • THE PROPOSED PROJECT WILL HAVE NO SIGNIFICANT IMPACT ON AIR OR WATER QUALITY. THERE ARE NO NATURAL FEATURES ON OR ADJACENT
- TO THE SITE THAT WILL BE IMPACTED BY THE PROPOSED RENOVATION WE ARE NOT AWARE OF ANY HISTORIC SITES OR STRUCTURES LOCATED ON THE SITE.

- SITE ANALYSIS • THE SITE IS PART OF A LARGER COMMERCIAL RETAIL CENTER. IT IS LOCATED AT THE WEST SIDE OF SOUTH MAPLE ROAD. THE SITE IS
- CURRENTLY VACANT BUT WAS PREVIOUSLY USED AS A RESTAURANT. • THE SOIL SURVEY OF WASHTENAW COUNTY INDICATES THAT THE PREDOMINANT SOIL TYPES ON SITE ARE URBAN LAND AND WAWASEE
- WE DO NOT BELIEVE THERE ARE ANY ENDANGERED SPECIES OR
- HABITATS LOCATED ON THIS PREVIOUSLY DEVELOPED SITE. • THIS SITE IS LOCATED IN AN AREA OF MINIMAL FLOOD HAZARD (ZONE X) AND OUTSIDE THE 100-YEAR FLOOD PLAIN PER FEMA COMMUNITY
- PANEL NUMBER 26161C0244E DATED 04/03/2012. • TWO LANDMARK TREES ARE LOCATED ON THE SITE. THEY WILL BE
- REMOVED AND REPLACEMENT TREES PROVIDED. • THERE ARE NO STEEP SLOPES LOCATED ON SITE AS DEFINED BY THE
- CITY OF ANN ARBOR ZONING ORDINANCE. • THERE ARE NO OPEN WATERCOURSES LOCATED ON THIS SITE. THE RECEIVING WATERS FOR THIS SITE IS THE WEST PARK FAIRGROUNDS
- THERE ARE NO WETLANDS LOCATED ON SITE
- THERE ARE NO WOODLANDS LOCATED ON SITE. • EXISTING BUILDINGS, WITH ASSOCIATED DRIVES AND PARKING AREAS,
- WILL REMAIN ADJACENT TO THIS SITE. • THE EXISTING CURB CUT ONTO SOUTH MAPLE ROAD WILL BE MODIFIED TO ACCOMMODATE THE PROPOSED BUILDING, AND THE EXISTING ACCESS POINT TO THE NORTH WILL BE RELOCATED. THE REMAINING CURB CUTS
- AND ACCESS POINTS SERVING THE SITE WILL NOT BE ALTERED. • THE SITE IS CURRENTLY SERVED BY EXISTING PUBLIC AND FRANCHISE UTILITIES. EXISTING WATER, SANITARY SEWER AND GAS ARE LOCATED WITHIN THE SOUTH MAPLE ROAD RIGHT-OF-WAY. OVERHEAD ELECTRIC
- LINES ARE LOCATED ALONG THE EAST SIDE OF THE SITE. • THE SITE SURFACE GENERALLY DRAINS FROM WEST TO EAST AND MOST LIKELY ULTIMATELY DISCHARGES INTO THE NORTH BRANCH OF FELLOWS CREEK. THIS SITE DOES NOT APPEAR TO RECEIVE DRAINAGE FROM

ADJACENT SITES.

• PLEASE REFERENCE THE TRAFFIC IMPACT STUDY PREPARED BY ROWE FOR ADDITIONAL TRAFFIC DATA RELATED TO THE PREVIOUS AND PROPOSED LAND USES.

PUBLIC SIDEWALK MAINTENANCE STATEMENT

 ALL SIDEWALKS SHALL BE KEPT AND MAINTAINED IN GOOD REPAIR BY THE OWNER OF THE LAND ADJACENT TO AND ABUTTING THE SAME. PRIOR TO THE ISSUANCE OF THE FINAL CERTIFICATE OF OCCUPANCY FOR THIS SITE, ALL EXISTING SIDEWALKS IN NEED OF REPAIR MUST BE REPAIRED IN ACCORDANCE WITH CITY STANDARDS.

TRASH AND RECYCLING COLLECTION

 TRASH AND RECYCLING WILL BE REMOVED FROM THE BUILDING AND DEPOSITED IN THE APPROPRIATE DUMPSTERS DAILY. TRASH AND RECYCLING WILL BE COLLECTED FROM THE DUMPSTERS ONCE WEEKLY.



N AN APPROXIMATE WAY ONLY AND HAVE NOT BEEN INDEPENDENTLY PRESERVE ANY AND ALL UNDERGROUND UTILITIES

CONSTRUCTION SITE SAFETY IS RESPONSIBILITY OF THE EXPECTED TO ASSUME ANY RESPONSIBILITY FOR SAFETY OF STRUCTURES, OR OF ANY OTHER

09.29.2020 REVISED PER SITE PLAN REVIEW #1

12.07.20
REVISED PER SITE PLAN REVIEW #3

RELATIONSHIPS - REPUTATION - RESULTS 800.525.6016

1051-19-9457 08/26/2020 |Checked by |Drawn by | Crew/Book

AR BROUWER COMPANY

CB CB KMW

LEWIS JEWELERS

300 S. MAPLE ROAD

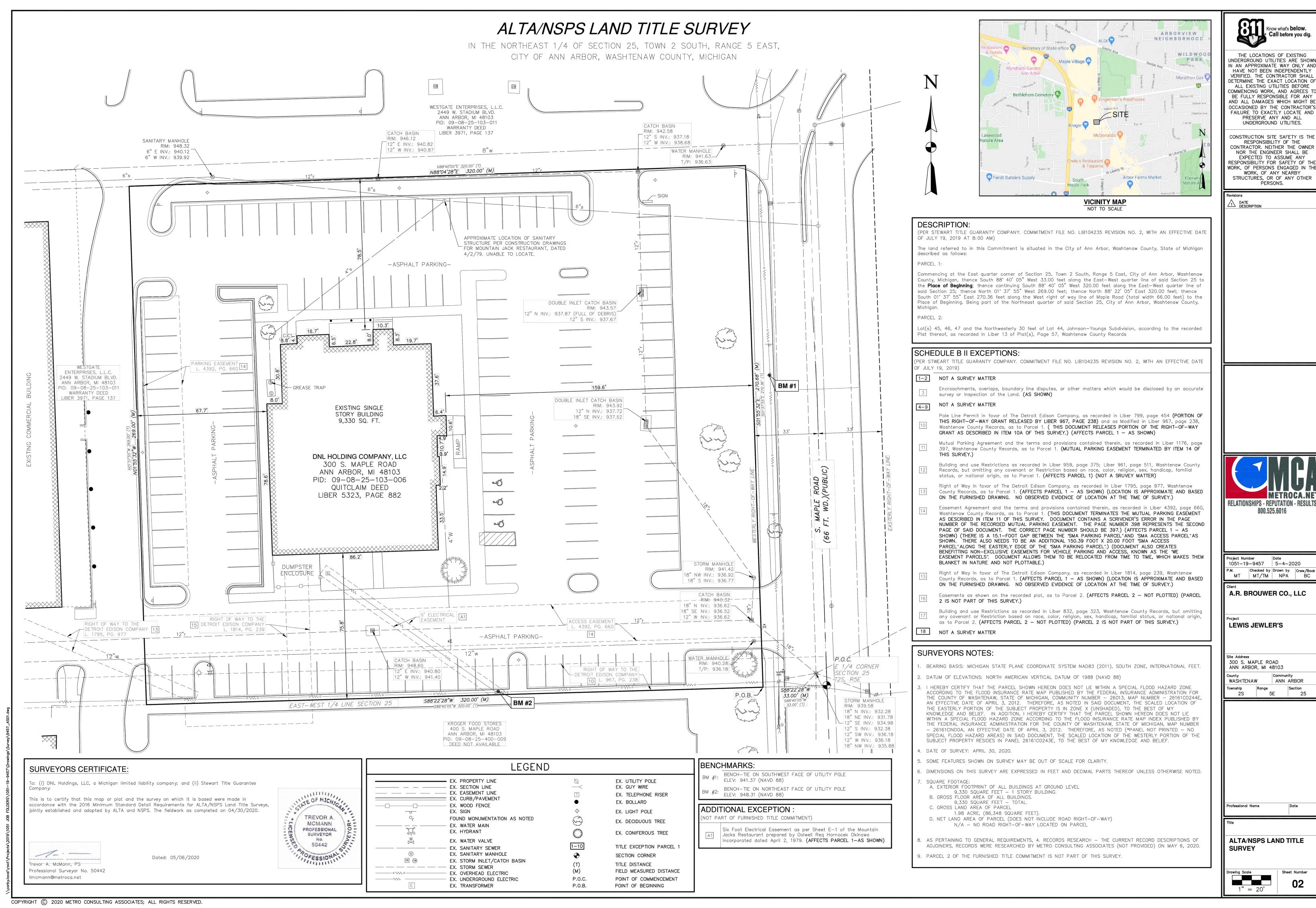
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ANN ARBOR, MICHIGAN 48103 WASHTENAW ANN ARBOR

CHRISTOPHER SUTTON, PE --

SITE PLAN **COVER SHEET**

Sheet Number



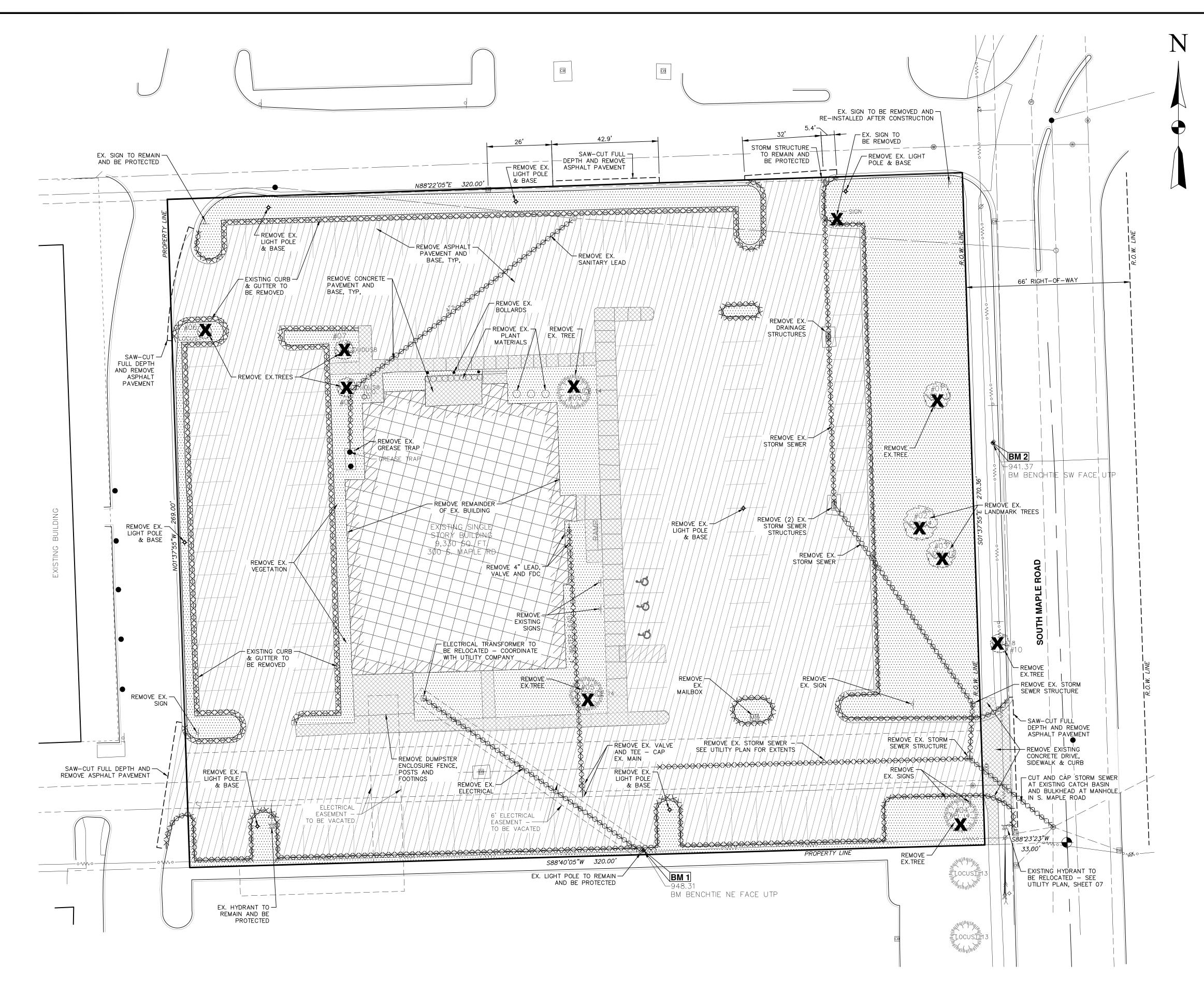
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N AN APPROXIMATE WAY ONLY AND HAVE NOT BEEN INDEPENDENTLY VERIFIED. THE CONTRACTOR SHALL DETERMINE THE EXACT LOCATION O ALL EXISTING UTILITIES BEFORE COMMENCING WORK, AND AGREES T BE FULLY RESPONSIBLE FOR ANY AND ALL DAMAGES WHICH MIGHT BE OCCASIONED BY THE CONTRACTOR'S FAILURE TO EXACTLY LOCATE AND

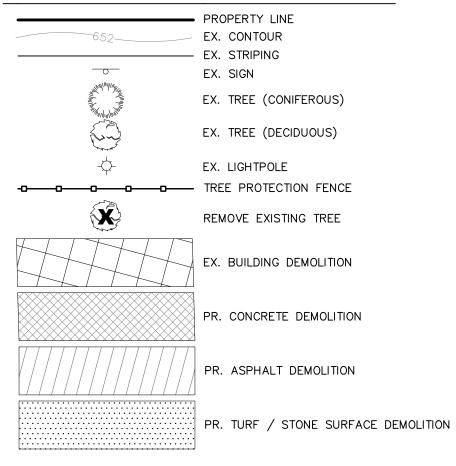
CONSTRUCTION SITE SAFETY IS THE CONTRACTOR. NEITHER THE OWNER NOR THE ENGINEER SHALL BE EXPECTED TO ASSUME ANY RESPONSIBILITY FOR SAFETY OF THE WORK, OF PERSONS ENGAGED IN THE

RELATIONSHIPS - REPUTATION - RESULTS

| Checked by | Drawn by | Crew/Book MT MT/TM NPA BC



LEGEND



DEMOLITION NOTES:

- THE CONTRACTOR SHALL BE RESPONSIBLE TO DEMOLISH OR RELOCATE ANY SITE FEATURES AS ACCORDING TO PLAN OR AS APPROPRIATE TO FACILITATE THE CONSTRUCTION OF THE PROPOSED IMPROVEMENTS.
- ALL WORK SHALL COMPLY WITH THE CITY OF ANN ARBOR STANDARDS AND SPECIFICATIONS.
- 3. CONTRACTOR IS RESPONSIBLE FOR PRESERVING LANDSCAPED AREAS THROUGHOUT CONSTRUCTION. ANY PLANTINGS, VEGETATION, OR GROUNDCOVER DAMAGED DURING CONSTRUCTION SHALL BE REPLACED WITH NEW TO MATCH EXISTING AT THE CONTRACTOR'S EXPENSE.
- 4. THE CONTRACTOR SHALL CALL 811 "MISS DIG" AT LEAST THREE WORKING DAYS PRIOR TO CONSTRUCTION.
- 5. A SOIL EROSION CONTROL PERMIT FROM THE AUTHORITY HAVING JURISDICTION IS REQUIRED AND SOIL EROSION CONTROL BMP'S SHALL BE IN PLACE PRIOR TO THE COMMENCEMENT OF DEMOLITION ACTIVITIES.
- 6. THE CONTRACTOR SHALL INDEPENDENTLY VERIFY THE LOCATION AND DEPTH OF EXISTING UTILITIES PRIOR TO DEMOLITION ACTIVITIES.
- 7. PRIOR TO REMOVING, RELOCATING, OR PERFORMING ANY WORK ON A UTILITY, THE CONTRACTOR SHALL COORDINATE WITH THE RESPECTIVE UTILITY OWNER.
- 8. ALL DEMOLISHED MATERIAL SHALL BE REMOVED FROM THE SITE AND DISPOSED OF IN A LEGALLY DESIGNATED DISPOSAL AREA. PERMITS AND FEES FOR DISPOSAL OF DEMOLITION MATERIAL SHALL BE OBTAINED AND PAID FOR BY THE CONTRACTOR.
- 9. PAVEMENTS TO BE REMOVED SHALL BE SAW CUT TO THE FULL DEPTH OF THE PAVEMENT. THE CONTRACTOR SHALL TAKE CARE TO MAINTAIN A CLEAN EDGE OF PAVEMENT.
- 10. ALL UNUSED SERVICE LEADS SHALL BE PERMANENTLY KILLED AT ITS RESPECTIVE MAIN

EXISTING CONDITIONS NOTES:

- 1. THIS PLAN IS BASED ON THE TOPOGRAPHIC SURVEY PREPARED BY METRO CONSULTING ASSOCIATES DATED FEBRUARY 13, 2020.
- 2. ELEVATIONS SHOWN HEREIN ARE BASED ON DATUM NATIONAL GEODETIC VERTICAL DATUM OF 1988 (NAVD88)
- 3. THE BEARING BASIS FOR THE SITE IS STATE PLANE, MICHIGAN SOUTH ZONE, INTERNATIONAL FEET.
- 4. THE PARCEL SHOWN IS WITHIN AN AREA OF MINIMAL FLOOD HAZARD (ZONE X) ACCORDING TO FEMA FLOOD INSURANCE RATE MAP, 26161CO244E
- 5. EXISTING UTILITIES ARE BASED ON FIELD OBSERVATIONS OF VISIBLE STRUCTURES IN CONJUNCTION WITH UTILITY MAPS OBTAINED AND/OR MARKED IN THE FIELD BY OTHERS. METRO CONSULTING ASSOCIATES (MCA) MAKES NO GUARANTEE AS TO THE ACCURACY OR COMPLETENESS OF THE EXISTING UTILITY INFORMATION SHOWN. ALL UTILITY LOCATIONS AND DEPTHS SHOULD BE INDEPENDENTLY VERIFIED PRIOR TO CONSTRUCTION. ANY DISCREPANCIES WITH THE PLANS SHOULD BE BROUGHT TO THE DESIGN PROFESSIONAL'S ATTENTION IMMEDIATELY.
- 6. THE EXISTING BUILDING HAS NO BASEMENT.

BENCHMARKS:

BM #1: BENCH TIE IN NORTHEAST FACE OF UTILITY POLE ELEVATION: 948.31' (NAVD88)

BM #2: BENCH TIE IN SOUTHWEST FACE OF UTILITY POLE ELEVATION: 941.37' (NAVD88)

EXISTING TREE LIST:

NO.	SIZE	SPECIES	STATUS
01	11" CAL.	CRAB APPLE TREE	TO BE REMOVED
02	14" CAL. (LM)	CRAB APPLE TREE	TO BE REMOVED
03	12" CAL. (LM)	CRAB APPLE TREE	TO BE REMOVED
04	14" CAL.	PINE TREE	TO BE REMOVED
05	14" CAL.	SPRUCE	TO BE REMOVED
06	3" CAL.	LOCUST TREE	TO BE REMOVED
07	8" CAL.	CRAB APPLE TREE	TO BE REMOVED
08	8" CAL.	CRAB APPLE TREE	TO BE REMOVED
09	14" CAL.	SPRUCE	TO BE REMOVED
10	8" CAL.	CRAB APPLE TREE	TO BE REMOVED
	(LM) DESIGNA	TES ANN ARBOR LAND	MARK TREE

Know what's below.
Call before you dig.

THE LOCATIONS OF EXISTING

UNDERGROUND UTILITIES ARE SHOWN IN AN APPROXIMATE WAY ONLY AND HAVE NOT BEEN INDEPENDENTLY VERIFIED. THE CONTRACTOR SHALL DETERMINE THE EXACT LOCATION OF ALL EXISTING UTILITIES BEFORE COMMENCING WORK, AND AGREES TO BE FULLY RESPONSIBLE FOR ANY AND ALL DAMAGES WHICH MIGHT BE OCCASIONED BY THE CONTRACTOR'S FAILURE TO EXACTLY LOCATE AND PRESERVE ANY AND ALL UNDERGROUND UTILITIES.

CONSTRUCTION SITE SAFETY IS THE RESPONSIBILITY OF THE CONTRACTOR. NEITHER THE OWNER NOR THE ENGINEER SHALL BE EXPECTED TO ASSUME ANY RESPONSIBILITY FOR SAFETY OF THE WORK, OF PERSONS ENGAGED IN THE WORK, OF ANY NEARBY STRUCTURES, OR OF ANY OTHER PERSONS.

08.27.2020
SITE PLAN SUBMISSION

09.29.2020
REVISED PER SITE PLAN REVIEW #1

10.22.2020 REVISED PER SITE PLAN REVIEW #2

12.07.20
REVISED PER SITE PLAN REVIEW #3

RELATIONSHIPS - REPUTATION - RESULTS 800.525.6016

AR BROUWER COMPANY

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LEWIS JEWELERS

Site Address 300 S. MAPLE ROAD ANN ARBOR, MICHIGAN 48103

WASHTENAW ANN ARBOR

Township Range Section

2S 5E 25

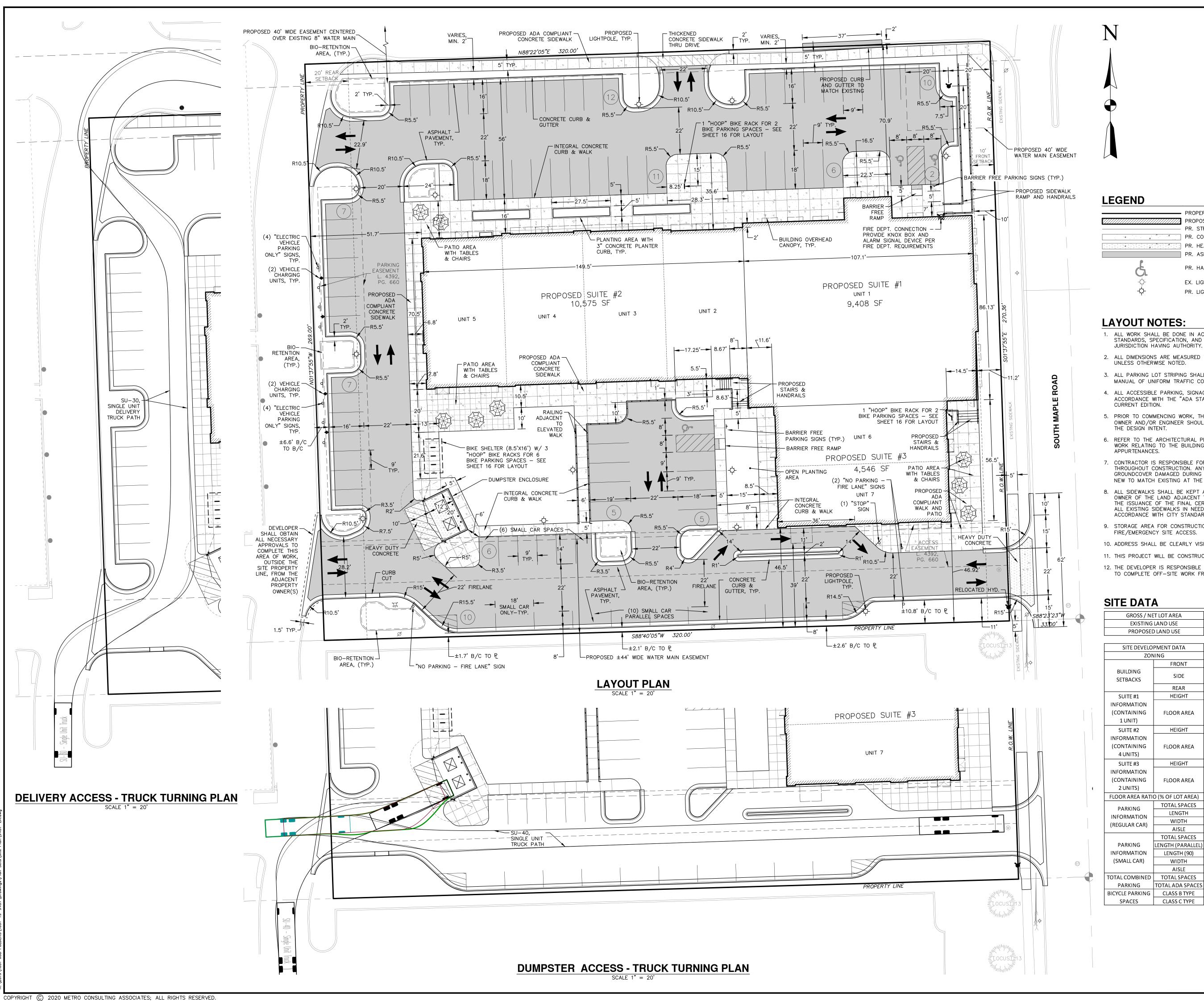
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CHRISTOPHER SUTTON, PE --

SITE PLAN

DEMOLITION PLAN

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10.22.2020 REVISED PER SITE PLAN REVIEW #2

12.07.20
REVISED PER SITE PLAN REVIEW #3

LAYOUT NOTES:

- 1. ALL WORK SHALL BE DONE IN ACCORDANCE WITH THE CURRENT STANDARDS, SPECIFICATION, AND GENERAL CONDITIONS OF THE
- 2. ALL DIMENSIONS ARE MEASURED TO THE PAINT LINE OR FACE OF CURB
- UNLESS OTHERWISE NOTED. 3. ALL PARKING LOT STRIPING SHALL BE IN ACCORDANCE WITH THE "MICHIGAN

PROPERTY LINE PROPOSED BUILDING

PR. STRIPING

PR. CONCRETE

PR. ASPHALT

EX. LIGHTPOLE

PR. LIGHTPOLE

PR. HEAVY-DUTY CONCRETE

PR. HANDICAP PARKING SYMBOL

- MANUAL OF UNIFORM TRAFFIC CONTROL DEVICES" 4. ALL ACCESSIBLE PARKING, SIGNAGE AND STRIPING SHALL BE IN
- ACCORDANCE WITH THE "ADA STANDARDS FOR ACCESSIBLE DESIGN", CURRENT EDITION.
- 5. PRIOR TO COMMENCING WORK, THE CONTRACTOR SHALL CONTACT THE OWNER AND/OR ENGINEER SHOULD THERE BE ANY FIELD CONFLICTS WITH THE DESIGN INTENT.
- 6. REFER TO THE ARCHITECTURAL PLANS FOR COORDINATING THE SCOPE OF WORK RELATING TO THE BUILDING ELEVATIONS, INTERIOR, AND APPURTENANCES.
- 7. CONTRACTOR IS RESPONSIBLE FOR PRESERVING LANDSCAPED AREAS THROUGHOUT CONSTRUCTION, ANY PLANTINGS, VEGETATION, OR GROUNDCOVER DAMAGED DURING CONSTRUCTION SHALL BE REPLACED WITH NEW TO MATCH EXISTING AT THE CONTRACTOR'S EXPENSE.
- 8. ALL SIDEWALKS SHALL BE KEPT AND MAINTAINED IN GOOD REPAIR BY THE OWNER OF THE LAND ADJACENT TO AND ABUTTING THE SAME. PRIOR TO THE ISSUANCE OF THE FINAL CERTIFICATE OF OCCUPANCY FOR THIS SITE, ALL EXISTING SIDEWALKS IN NEED OF REPAIR MUST BE REPAIRED IN ACCORDANCE WITH CITY STANDARDS.
- 9. STORAGE AREA FOR CONSTRUCTION MATERIALS SHALL NOT INTERFERE WITH FIRE/EMERGENCY SITE ACCESS.
- 10. ADDRESS SHALL BE CLEARLY VISIBLE WHEN APPROACHING THE BUILDING.
- 11. THIS PROJECT WILL BE CONSTRUCTED/COMPLETED IN A SINGLE PHASE.
- 12. THE DEVELOPER IS RESPONSIBLE FOR OBTAINING NECESSARY APPROVALS TO COMPLETE OFF-SITE WORK FROM THE NECESSARY PROPERTY OWNER(S).

SITE DATA

	GROSS / NE	T LOT AREA	1.98 AC							
		LAND USE	VACANT RESTAURANT							
	PROPOSEI	D LAND USE	COMMERCIAL							
		l								
	SITE DEVELO	PMENT DATA	EXISTING	REQUIRED	PROPOSED					
	ZON	NING	C3	C3	С3					
		FRONT	159.64' EAST	10'	10 EAST					
	BUILDING SETBACKS	SIDE	78.58' NORTH 75.95' SOUTH	0'	73' NORTH 47' SOUTH					
		REAR	67.66' WEST	20'	53' WEST					
	SUITE #1	HEIGHT	+/-28'/1-STORY	55 FT/4 STORIES	28'/1-STORY					
	INFORMATION (CONTAINING 1 UNIT)	FLOOR AREA	+/-9330 SF	6,000 SF	9,408 SF					
	SUITE #2	HEIGHT		55 FT/4 STORIES	28'/1-STORY					
	INFORMATION (CONTAINING 4 UNITS)	FLOOR AREA		6,000 SF	10,575 SF					
	SUITE #3	HEIGHT		55 FT/4 STORIES	28'/1-STORY					
ij	INFORMATION (CONTAINING 2 UNITS)	FLOOR AREA		6,000 SF	4,546 SF					
	FLOOR AREA RATI	O (% OF LOT AREA)		MAX. 200	28.4%					
	PARKING	TOTAL SPACES		(SEE BELOW)	65					
	INFORMATION	LENGTH		16' / 18' (MIN)	16' / 18'					
	(REGULAR CAR)	WIDTH		9' (MIN)	9'					
	(NEGOLAN CAN)	AISLE		22' (MIN)	22' MIN.					
I		TOTAL SPACES		(SEE BELOW)	16					
	PARKING	LENGTH (PARALLEL)		18' (MIN)	18'					
	INFORMATION	LENGTH (90)		14' / 16' (MIN)	14'					
	(SMALL CAR)	WIDTH		8' (MIN)	8'					
		AISLE		20' (MIN)	22'					
C	TOTAL COMBINED	TOTAL SPACES	142	79 (MIN.)*	81					
	PARKING	TOTAL ADA SPACES	3	3 (MIN.)	3					
	BICYCLE PARKING	CLASS B TYPE	0	4.5*	6					
	SPACES	CLASS C TYPE	0	4.5*	4					



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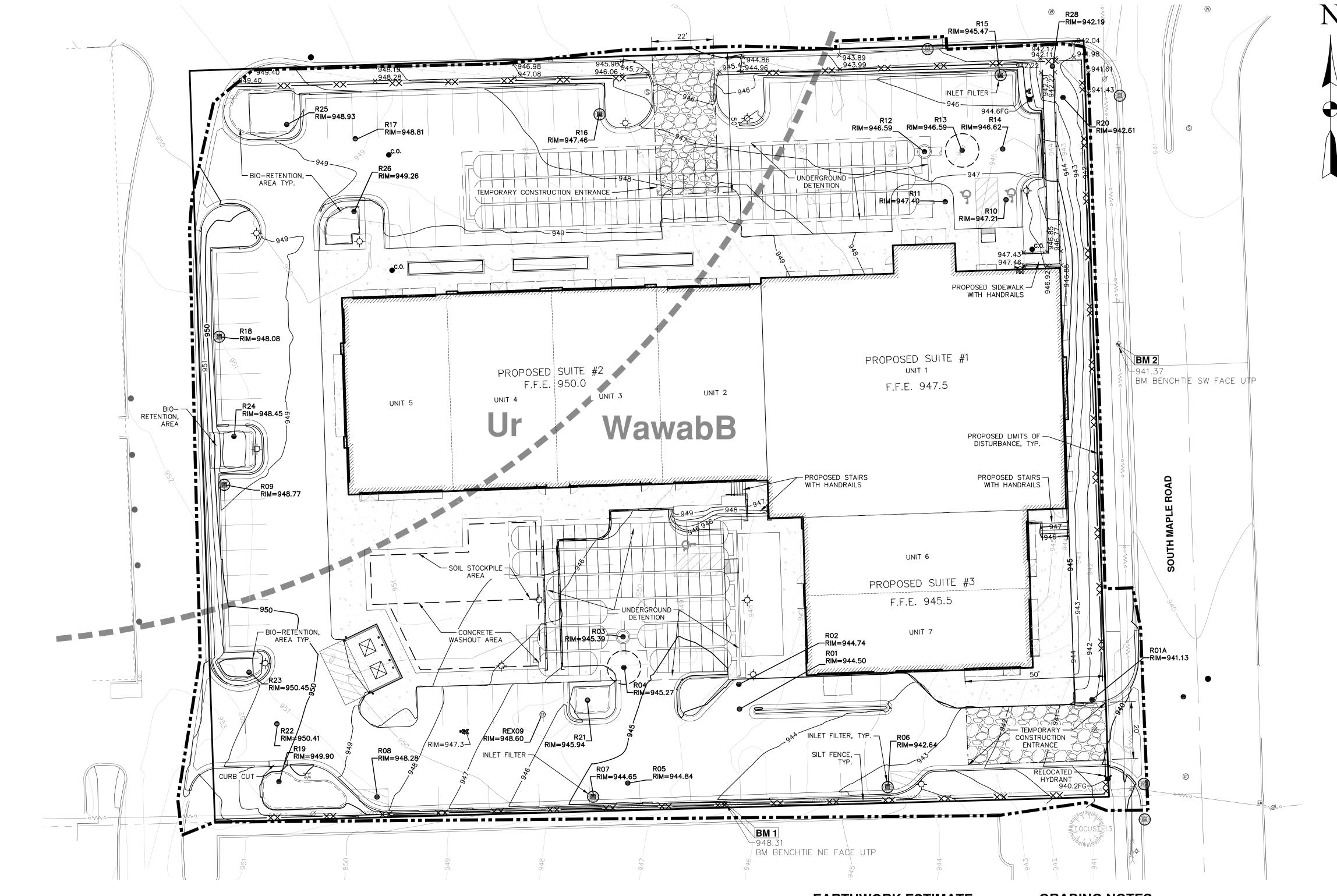
LEWIS JEWELERS

ite Address 300 S. MAPLE ROAD ANN ARBOR, MICHIGAN 48103

WASHTENAW ANN ARBOR Fownship Range Section 25

CHRISTOPHER SUTTON, PE --

SITE PLAN LAYOUT PLAN



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	EX. CUI	RB/PAVEMENT
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<u></u>		DENTIFIED MANHOL
Ø		AFFIC SIGNAL POLE
<u> </u>	EX. UTI EX. GU'	LITY POLE Y WIRF
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BENCHMARKS:

BM #1: BENCH TIE IN NORTHEAST FACE OF UTILITY POLE ELEVATION: 948.31' (NAVD88)

——— SILT FABRIC FENCE

SOIL TYPE DESIGNATION

BM #2: BENCH TIE IN SOUTHWEST FACE OF UTILITY POLE ELEVATION: 941.37' (NAVD88)

EARTHWORK ESTIMATE:

- THE ESTIMATED COST OF EARTHWORK IS \$125,000
- ESTIMATED CUT = 2,177 CY ESTIMATED FILL = 973 CY
- THE ESTIMATED COST OF SITE STABILIZATION IS \$5,300

GRADING NOTES:

- 1. THE PROPOSED CONTOURS AND SLOPES ARE APPROXIMATE AS SHOWN.
- 2. ALL PROPOSED SPOT GRADES SHOWN AT THE EDGE OF METAL OR TOP EDGE OF WALK, UNLESS OTHERWISE NOTED. NOTATION CODES ARE AS FOLLOWS: FF=FINISHED FLOOR, FG=FINISHED GRADE, TP=TOP OF PAVEMENT, RIM=TOP OF GRATE OR MANHOLE, TW=TOP OF WALL, BW=BOTTOM OF WALL, TC=TOP OF CURB.
- 3. ANY SOIL RETAINING SYSTEM OR WALL IF REQUIRED SHALL BE DESIGNED BY A QUALIFIED STRUCTURAL OR GEOTECHNICAL ENGINEER. A DETAIL OF THE RETAINING WALL WITH CALCULATIONS TO BE SUBMITTED TO THE REVIEW AGENCY AS REQUIRED.
- 4. WHEN PAVED AREAS ARE EXCAVATED, ASPHALT AREAS SHALL BE SAW-CUT AND REMOVED TO A DISTANCE EQUAL TO THE DEPTH OF EXCAVATION. CONCRETE SHALL BE REMOVED TO THE FIRST JOINT PAST THE DISTANCE EQUAL TO THE DEPTH OF EXCAVATION. COMPACTED SAND BACKFILL IS REQUIRED IN ALL EXCAVATIONS WITHIN A 1:1 SLOPE OF EXISTING OR PROPOSED PAVEMENT.
- 5. CONTRACTOR SHALL BE RESPONSIBLE TO ENSURE POSITIVE DRAINAGE REMAINS DURING AND IN POST CONSTRUCTION CONDITIONS AND NO ADVERSE IMPACTS WILL OCCUR TO NEIGHBORING PROPERTIES DURING AND AFTER COMPLETION OF CONSTRUCTION.
- 6. ALL SIDEWALK PLANNED TO BE ACCESSIBLE SHALL BE CONSTRUCTED WITH CROSS SLOPES LESS THAN 2%.
- 7. ALL SIDEWALK CURB RAMPS SHALL BE CONSTRUCTED PER THE LATEST MICHIGAN DEPARTMENT OF TRANSPORTATION DETAILS AND IN COMPLIANCE WITH THE AMERICAN WITH DISABILITIES ACT.

- 8. CONTRACTOR SHALL ADJUST ALL EXISTING MANHOLE/CATCH BASIN RIM ELEVATIONS AFFECTED BY THE PROPOSED WORK. WHERE NECESSARY, DUE TO EXCESSIVE ELEVATION CHANGE, CONTRACTOR SHALL RECONSTRUCT THE EXISTING STRUCTURE.
- 9. CONTRACTOR SHALL REFER TO THE GEOTECHNICAL REPORT FOR RECOMMENDATIONS ON BACKFILL, TRENCHING, GROUND WATER CONDITIONS, ETC. UNSUITABLE MATERIAL, IF ENCOUNTERED, BENEATH THE BUILDING ADDITION OR PROPOSED PAVEMENT, SHALL BE REMOVED ENTIRELY AND REPLACED WITH ENGINEERED FILL. AREAS REQUIRING SUBSTANTIAL REMEDIATION SHALL BE BROUGHT TO THE OWNER AND THE OWNER'S ENGINEER FOR FURTHER RECOMMENDATION.
- 10. ALL GRADES ARE TOP OF WALK OR TOP OF PAVEMENT UNLESS OTHERWISE SPECIFIED.
- 11. PARKING SPACES AND REQUIRED BARRIER FREE SIGNAGE SHALL COMPLY WITH THE MICHIGAN BUILDING CODE.
- 12. ALL CONSTRUCTION SHALL COMPLY WITH THE CITY OF ANN ARBOR STANDARDS AND SPECIFICATIONS.
- 13. ALL PROPOSED PAVEMENT MARKINGS TO BE PAINTED TO CURRENT INDUSTRY STANDARDS.
- 14. ADA PARKING STALLS AND AISLES SHALL NOT EXCEED 2% SLOPE IN ALL DIRECTIONS
- 15. INTERNAL AND EXTERNAL STREETS SHALL BE CLEANED OF ANY TRACKED MUD IMMEDIATELY FOLLOWING EACH MUD-TRACKING OCCURRENCE.



THE LOCATIONS OF EXISTING UNDERGROUND UTILITIES ARE SHOWN IN AN APPROXIMATE WAY ONLY AND HAVE NOT BEEN INDEPENDENTLY VERIFIED. THE CONTRACTOR SHALL DETERMINE THE EXACT LOCATION OF ALL EXISTING UTILITIES BEFORE COMMENCING WORK, AND AGREES T BE FULLY RESPONSIBLE FOR ANY AND ALL DAMAGES WHICH MIGHT BE OCCASIONED BY THE CONTRACTOR'S FAILURE TO EXACTLY LOCATE AND PRESERVE ANY AND ALL UNDERGROUND UTILITIES.

CONSTRUCTION SITE SAFETY IS THE RESPONSIBILITY OF THE CONTRACTOR. NEITHER THE OWNER NOR THE ENGINEER SHALL BE EXPECTED TO ASSUME ANY RESPONSIBILITY FOR SAFETY OF THE WORK, OF PERSONS ENGAGED IN THI WORK, OF ANY NEARBY STRUCTURES, OR OF ANY OTHER PERSONS.

08.27.2020 SITE PLAN SUBMISSION

- 09.29.2020
 REVISED PER SITE PLAN REVIEW #1
- 10.22.2020 REVISED PER SITE PLAN REVIEW #2
- 12.07.20
 REVISED PER SITE PLAN REVIEW #3

RELATIONSHIPS - REPUTATION - RESULTS

800.525.6016

1051-19-9457 08/26/2020 | Checked by Drawn by | Crew/Book

AR BROUWER COMPANY

CB | CB | KMW

LEWIS JEWELERS

300 S. MAPLE ROAD

ANN ARBOR, MICHIGAN 48103

WASHTENAW ANN ARBOR 2S 5E 25

HRISTOPHER SUTTON, PE --

SITE PLAN

GRADING & SOIL EROSION CONTROL PLAN

Sheet Number

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RECEIVING WATER: WEST PARK FAIRGROUNDS DRAIN

SITE SOILS INFORMATION:

Ur: URBAN LAND WawabB: WAWASEE LOAM, 2 TO 6 PERCENT SLOPE

PER THE NATIONAL RESOURCES CONSERVATION SERVICE (NRCS) SOIL SURVEY

APPROXIMATE AREA OF DISTURBANCE: 2 ± ACRES

PERSON RESPONSIBLE FOR ON-SITE SOIL EROSION CONTROL CONTRACTOR: TBD PHONE: TBD FAX: TBD

APPROXIMATE CONSTRUCTION SCHEDULE IS AS FOLLOWS:

03/2021 PROJECT START DATE

03/2021 TEMPORARY EROSION CONTROL MEASURES INSTALLED

03/2021 GRAVEL CONSTRUCTION ENTRANCE INSTALLED

GENERAL SOIL EROSION CONTROL NOTES:

03/2021 DEMOLITION ACTIVITIES

04/2021 DETENTION FACILITIES INSTALLED 04/2021 UTILITIES / STORM SEWER / INLETS FILTERS INSTALLED

05/2021 FIRST COARSE OF PAVEMENT, CURB AND GUTTER INSTALLED

05/2021 BUILDING CONSTRUCTION 08/2021 FINAL COARSE PAVING INSTALLED

08/2021 FINAL GRADING / SEEDING

09/2021 CATCH BASINS CLEANED 10/2021 PERMANENT EROSION CONTROL MEASURES IN PLACE 11/2021 TEMPORARY EROSION CONTROL MEASURES REMOVED

12/2021 PROJECT END DATE

TEMPORARY STABILIZATION											
AREA REQUIRING TEMPORARY STABILIZATION	TIME FRAME TO APPLY EROSION CONTROLS										
ANY DISTURBED AREASE WITHIN 50 FEET OF A STREAM	WITHIN TWO DAYS OF THE MOST RECENT DISTURBANCE IF										
AND NOT AT FINAL GRADE	THE AREA WILL REMAIN IDLE FOR MORE THAN 21 DAYS										
FOR ALL CONSTRUCTION ACTIVITIES, ANY DISTURBED	WITHIN SEVEN DAYS OF THE MOST RECENT DISTURBANCE										
AREAS THAT WILL BE DORMANT FOR MORE THAN 21 DAYS	WITHIN THE AREA										
BUT LESS THAN ONE YEAR, AND NOT WITHIN 50 FEET OF A											
STREAM	FOR RESIDENTIAL SUBDIVISIONS, DISTURBED AREAS										
	MUST BE STABILIZED AT LEAST SEVEN DAYS PRIOR TO THE										
	TRANSFER OF PERMIT COVERAGE FOR THE INDIVIDUAL										
	LOT(S)										
DISTURBED AREAS THAT WILL BE IDLE OVER THE WINTER	PRIOR TO THE ONSET OF WINTER WEATHER										
PERMANENT	STABLIZATION										
AREA REQUIRING PERMANENT STABILIZATION	TIME FRAME TO APPLY EROSION CONTROLS										
ANY AREAS THAT WILL LIE DORMANT FOR ON YEAR OR	WITHIN SEVEN DAYS OF THE MOST RECENT DISTURBANCE										
MORE											
ANY AREAS WITHIN 50 FEET OF A STREAM AND AT FINAL	WITHIN TWO DAYS OF REACHING FINAL GRADE										
GRADE											

THAT AREA

WITHIN SEVEN DAYS OF REACHING FINAL GRADE WITHIN

WOODEN POST DRIVEN INTO -

GROUND A MINIMUM OF 12".

MAX. OF 6.5' SPACING O.C.

WRAP END POSTS TOGETHER AT LEAST ONE REVOLUTION

WHEN SPLICING SECTIONS OF

INSTALLATION DETAIL

SILTFENCE.

DIRECTION

EXISTING: GROUND

SHEET FLOW-

STABILIZATION SPECIFICATIONS

"TEMPORARY SEEDING" NO AREA FOR WHICH GRADING HAS BEEN COMPLETED SHALL BE LEFT UNSEEDED OR UNMULCHED FOR LONGER THAN 21 DAYS. IF PERMANENT SEED IS NOT APPLIED AT THIS TIME, TEMPORARY SEEDING SHALL BE DONE AT THE FOLLOWING RATES.

MARCH 1 TO AUGUST 15 SEED: OATS 2 LBS./1,000 SQ.FT. FERTILIZER: (12:12:12) 12 1/2 LBS./1,000 SQ.FT. MULCH: (STRAW OR HAY) 2 TONS/ACRE AUGUST 15 TO NOVEMBER 1 SEED: ANNUAL RYE 2 LBS./1,000 SQ.FT. FERTILIZER: (12:12:12) 12 1/2 LBS./1,000 SQ.FT. 2 TONS/ACRE MULCH: (STRAW OR HAY)

NOVEMBER 1 TO MARCH 1

MULCH (ONLY): (STRAW OR HAY)

RATES OF APPLICATION OF ITEM 659:

"PERMANENT SEEDING" SHALL BE DONE BETWEEN MARCH 15 AND OCTOBER 15. IF SEEDING IS DONE BETWEEN OCTOBER 15 AND MARCH 15, IT SHALL BE CLASSIFIED AS "TEMPORARY SEEDING" PERMANET SEED SHALL BE 40% KENTUCKY BLUEGRASS, 40% CREEPING RED FESCUE, 20% ANNUAL RYEGRASS. PERMANENT SEEDING SHALL CONSIST OF FERTILIZING, WATERING AND SEEDING RATES INDICATED UNDER ITEM 659. SEEDING SHALL BE APPLIED WITHIN TWO DAYS AFTER FINAL GRADING OR FOLLOWING SEED BED PREPARATION.

2 TONS/ACRE

4 LBS./1,000 SQ.FT. FERTILIZER: (12:12:12) 20 LBS./1,000 SQ.FT. MULCH: (STRAW OR HAY) 2 TONS/ACRE

ESTIMATED COST OF SITE STABILIZATION: \$5,300

SILT FENCE GEOTEXTILE

FASTENED TO POSTS ON

_6"x6" ANCHOR TRENCH.

EXTEND ACROSS BOTTOM

OF TRENCH AND THEN

UPHILL SIDE TOWARDS

EARTH DISTURBANCE.

FILTER FABRIC TO

BACKFILLED WITH

COMPACTED EARTH.

BAG DETAIL

FABRIC SECURELY

SOIL EROSION CONTROL MAINTENANCE TASKS AND SCHEDULE

DURING CONSTRUCTION TO BE PERFORMED BY CONTRACTOR SLUBNOGWOO TASKS	PAVED AREAS	PERVIOUS AREAS	RIP-RAP/SILT FENCE	STORM DRAINAGE SYSTEM	CATCH BASIN FILTERS	CATCH BASIN INLET CASTIN	OUTLET STRUCTURE	BIO-FILTRATION AREA	DETENTION AREA	SCHEDULE
INSPECT FOR SEDIMENT ACCUMULATION	X		Х	X	Х		Х	Χ	Х	WEEKLY
REMOVAL OF SEDIMENT ACCUMULATION	X		Х	Х	Х		X	Χ	X	AS NEEDED* AND PRIOR TO TURNOVER
INSPECT FOR FLOATABLES AND DEBRIS				Х	Х	Х	Х	Χ	Х	QUARTERLY
CLEANING FOR FLOATABLES AND DEBRIS				Х	Х	Х	Х	Χ	Х	QUARTERLY AND AT TURNOVER
INSPECTION FOR EROSION		X	Х							WEEKLY
REESTABLISH PERMANENT VEGETATION ON ERODED SLOPES	ŝ	X								AS NEEDED* AND PRIOR TO TURNOVER
CLEAN DRIVES AND PARKING LOTS	X									WEEKLY OR AS DETERMINED BY PERMITTING AGENCY
WATER DISTURBED AREAS TO PROVIDE DUST CONTROL	AL	L DI	STU	RBE	D A	REA	S O	F S	SITE	AS NEEDED
INSPECT STRUCTURAL ELEMENTS DURING WET WEATHER AND COMPARE TO AS-BUILT PLANS (BY A PROFESSIONAL ENGINEER REPORTING TO THE OWNER)			X	×			X	Χ	X	ANNUALLY AND AT TURNOVER
MAKE ADJUSTMENTS OR REPLACEMENTS AS DETERMINED			X	Х			X	Χ	Х	AS NEEDED

* "AS NEEDED" MEANS WHEN SEDIMENT HAS ACCUMULATED TO A MAXIMUM OF ONE FOOT DEPTH

LONG TERM STORM WATER MAINTENANCE PLAN & BUDGET

TO BE PERFORMED BY OWNER OR OWNER'S REP.

SL N N N N N N N N N N N N N N N N N N N	PAVED AREAS	PERVIOUS AREAS	1 ~	CATCH BASIN SUMPS	CATCH BASIN INLET CO	ION BASIN	OUTLET CONTROL STR	BIO-FILTRATION AREA	ENTION AREA		SCHEDULE	ANNUAL COST
INSPECT FOR SEDIMENT ACCUMULATION	X		X	Х	X	Х	Х	Χ	Х		SEMI-ANUALLY/AS NEEDED*	\$100.00
REMOVAL OF SEDIMENT ACCUMULATION	Х		Х	Х	X	Х	Х	Х	Х		5-10 YRS/AS NEEDED*	\$200.00
INSPECT FOR FLOATABLES AND DEBRIS			Х	Х		Х	Х	Χ	Х		ANNUALLY	\$100.00
CLEANING FOR FLOATABLES AND DEBRIS			X	Х		Х	Х	Χ	Х		ANNUALLY	\$300.00
INSPECTION FOR EROSION		Х				Х		Χ	Х		ANNUALLY/AFTER MAJOR STORMS	\$100.00
REESTABLISH PERMANENT VEGETATION ON ERODED SLOPES		Х				Х		Χ	Х		AS NEEDED	\$300.00
CLEAN DRIVES AND PARKING LOTS	Х										ANNUALLY	\$500.00
MOWING		X									0-2 TIMES PER YEAR	\$400.00
INSPECT STRUCTURAL ELEMENTS DURING WET WEATHER AND COMPARE TO AS-BUILT PLANS (BY A PROFESSIONAL ENGINEER REPORTING TO THE OWNER)			X			×	Х	Χ	Х		ANNUALLY	\$100.00
MAKE ADJUSTMENTS OR REPLACEMENTS AS DETERMINED BY ANNUAL WET WEATHER INSPECTION			Х			Х	Х	Х	Х		AS NEEDED	\$100.00
KEEP RECORDS OF ALL INSPECTIONS AND MAINTENANCE AGE TO PROPERTY OWNER	CTIV	/ITIE	S A	ND	RE	POR	?T				ANNUALLY	\$50.00
KEEP RECORDS OF ALL COSTS FOR INSPECTIONS, MAINTEN. REPORT TO PROPERTY OWNER	ANC	E A	ND	RE	PAIF						ANNUALLY	\$50.00
PROPERTY OWNER REVIEWS COST EFFECTIVENESS OF THE F MAINTENANCE PROGRAM AND MAKES NECESSARY ADJUSTM			TAT	IVE							ANNUALLY	\$50.00
OWNER TO HAVE A PROFESSIONAL ENGINEER CARRY OUT E INSPECTIONS UPON IDENTIFICATION OF SEVERE PROBLEMS	NCY	/							AS NEEDED	\$150.00		
* "AS NEEDED" MEANS WHEN SEDIMENT HAS ACCUMULATED) T(Λ Δ	MA	VIV	MI IN	ΩF	<u> </u>	IF F	-00	T D	FPTH	•

"AS NEEDED" MEANS WHEN SEDIMENT HAS ACCUMULATED TO A MAXIMUM OF ONE FOOT DEPTH

- CONCRETE WASHOUT

PLAN, COMPLETED APPLICATION AND CHECKLIST FORMS, PAY ALL FEES AND POST AN EROSION CONTROL PERFORMANCE BOND, AS REQUESTED. 3. CONSTRUCTION OPERATION SHALL BE SCHEDULED AND PERFORMED SO THAT PREVENTATIVE SOIL EROSION CONTROL MEASURES ARE IN PLACE PRIOR TO EXCAVATION IN CRITICAL AREAS AND TEMPORARY STABILIZATION MEASURES ARE IN PLACE IMMEDIATELY FOLLOWING BACKFILLING

ANY OTHER AREAS AT FINAL GRADE

4. SPECIAL PRECAUTIONS WILL BE TAKEN IN THE USE OF CONSTRUCTION EQUIPMENT TO PREVENT SITUATIONS THAT PROMOTE EROSION.

1. THE CONTRACTOR SHALL CONFORM TO PART 91 OF ACT 451 OF THE PUBLIC ACTS OF 1994; EROSION AND SEDIMENT CONTROL OF RUNOFF DURING CONSTRUCTION (AS AMENDED) AND

2. PRIOR TO ANY EARTH CHANGE, THE DEVELOPER SHALL SUBMIT A DETAILED EROSION CONTROL

CURRENT LOCAL ORDINANCES FOR EROSION AND SEDIMENTATION CONTROL.

- 5. CLEANUP WILL BE DONE IN A MANNER TO ENSURE THAT EROSION CONTROL MEASURES ARE
- 6. NATIONAL POLLUTANT DISCHARGE ELIMINATION SYSTEM (NPDES) PERMIT FOR STORM WATER DISCHARGE FROM THE CONSTRUCTION ACTIVITIES IS REQUIRED PRIOR TO ANY EARTH CHANGE.
- 7. THE CONTRACTOR IS REQUIRED TO KEEP A COPY OF THE APPROVED SOIL EROSION AND SEDIMENT CONTROL PLAN AND PERMIT AT THE CONSTRUCTION SITE.

PERMANENTLY STABILIZED AS DETERMINED BY THE SOIL EROSION INSPECTOR.

- 8. ALL SOIL EROSION CONTROL PRACTICES TO BE INSTALLED PRIOR TO ANY MAJOR SOIL DISTURBANCE, OR IN THEIR PROPER SEQUENCE, AND MAINTAINED UNTIL SUCH MEASURES ARE
- 9. DURING CONSTRUCTION, ANY ADDITIONAL CONTROL MEASURES AS DEEMED NECESSARY TO PREVENT EROSION OR CONTROL SEDIMENT BEYOND THOSE MEASURES SHOWN ON THE APPROVED PLANS SHALL BE INSTALLED OR EMPLOYED AT THE DIRECTION OF THE LOCAL JURISDICTION OR THE SOIL EROSION INSPECTOR.
- 10. TEMPORARY AND PERMANENT STABILIZATION SHALL BE CONDUCTED PER THE TABLE BELOW.
- 11. TRENCH AND GROUND WATER MUST PASS THROUGH A SEDIMENT POND, FILTER BAG OR OTHER COMPARABLE METHOD PRIOR TO DISCHARGING FROM THE SITE, IF USING A FILTER BAG, IT SHOULD BE PLACED ON FLAT GROUND TO ENSURE EFFICIENCY. THE FILTER BAG SHOULD BE LOCATED A SUFFICIENT DISTANCE FROM THE EXISTING WATERCOURSE OR WETLAND TO ALLOW PROPER SETTLING OR FILTERING THROUGH NATURAL VEGETATION. DEWATERING DISCHARGE SHALL BE MONITORED FOR ANY EROSIVE CONDITIONS. IF EROSION OCCURS, DEWATERING OPERATIONS MUST CEASE AND THE ERODED AREA MUST BE STABILIZED IMMEDIATELY, AND MAY RESUME ONLY AFTER STABILIZATION IS COMPLETE.
- 12. ANY TEMPORARY SOILS STOCKPILE SHALL OCCUR WITHIN THE LIMITS OF THE SILT FENCE. STOCKPILES TO BE GRADED TO A MAXIMUM OF 3:1 SIDE SLOPE.
- 13. A CONCRETE WASHOUT AREA SHALL BE DESIGNATED AND UTILIZED AS NECESSARY, CONCRETE TRUCKS ARE NOT PERMITTED TO WASH OUT DIRECTLY INTO STORM SEWERS, STREAMS, OR
- 14. GRADING SHALL BE DONE AS TO NOT DISRUPT THE STORM WATER FROM ADJACENT
- 15. NO SOLID OR LIQUID WASTE, INCLUDING BUILDING MATERIALS OR THEIR PACKAGING, SHALL DISCHARGE INTO STORM WATER RUN OFF.
- 16. CONTAMINATED SOILS WHERE CONSTRUCTION SITE CHEMICALS HAVE BEEN SPILLED MUST BE REMOVED FROM THE SITE AND DISPOSED OF IN ACCORDANCE WITH FEDERAL, STATE, AND

MAINTENANCE REQUIREMENTS:

- 1. ALL BMPS MUST BE MAINTAINED IN A FUNCTIONAL CONDITION UNTIL ALL UPSLOPE AREAS THEY CONTROL ARE PERMANENTLY
- 2. QUALIFIED PERSONNEL (PROVIDED BY THE DEVELOPER) MUST INSPECT ALL BMPS AT LEAST ONCE EVERY 7 DAYS AND WITHIN 24 HOURS OF A 0.5" OR GREATER RAINFALL WITHIN ANY 24-HOUR PERIOD AND DETERMINE IF THE SWP3 HAS BEEN PROPERLY IMPLEMENTED.
- 3. WRITTEN REPORTS SUMMARIZING INSPECTION RESULTS MUST BE MADE AVAILABLE UPON REQUEST. REPORTS MUST INCLUDE: DATE OF INSPECTION, NAME AND QUALIFICATIONS OF THE INSPECTOR. WEATHER CONDITIONS, LOCATIONS WHERE IN-STREAM OR OFF-SITE SEDIMENTATION WAS OBSERVED, LOCATION OF BMPS NEEDING MAINTENANCE, LOCATIONS OF BMPS FAILING TO OPERATE CORRECTLY OR PROVIDE ADEQUATE PROTECTION, OR LOCATION OF AREAS IN NEED OF ADDITIONAL BMPS NOT IN PLACE AT THE TIME OF INSPECTION.
- 4. THE REPORTS MUST IDENTIFY INCIDENTS OF NON-COMPLIANCE WITH THE NPDES PERMIT. WHERE A REPORT DOES NOT IDENTIFY INCIDENTS OF NON-COMPLIANCE, THE REPORT MUST CONTAIN A CERTIFICATION THAT THE SITE IS IN COMPLIANCE AT THE TIME OF
- 5. MAINTENANCE OR REPAIR OF BMPS MUST BE COMPLETED WITHIN 3 DAYS OF THE DATE OF THE INSPECTION THAT REVEALED THEY WERE DEFICIENT. FOR SEDIMENT PONDS, REPAIR OR MAINTENANCE IS REQUIRED WITHIN 10 DAYS OF THE INSPECTION.
- 6. WHEN INSPECTIONS REVEAL THAT A BMP IS NOT EFFECTIVE AND THAT ANOTHER, MORE APPROPRIATE BMP IS REQUIRED, THE SWP3 MUST BE AMENDED AND THE MORE APPROPRIATE BMP MUST BE INSTALLED WITHIN 10 DAYS OF THE INSPECTION THAT REVEALED
- 7. WHEN THE INSPECTION REVEALS THAT A BMP DEPICTED ON THE SWP3 HAS NOT BEEN INSTALLED, BUT IS REQUIRED TO PROVIDE ADEQUATE CONTROL AT THE SITE, IT MUST BE INSTALLED PRIOR TO THE NEXT STORM EVENT, WHICH PRODUCES RUNOFF, BUT IN NO CASE LATER THAN 10 DAYS FROM THE DATE OF INSPECTION, WHICH REVEALED THE DEFICIENCY.
- 8. THE REPORTS MUST BE MAINTAINED FOR THREE (3) YEARS FOLLOWING THE SUBMITTAL OF A NOTICE OF TERMINATION.

MAINTENANCE NOTES:

- 1. ALL DIRT AND MUD TRACKED ONTO ROADS DUE TO CONSTRUCTION SHALL BE REMOVED ON A DAILY BASIS BY THE CONTRACTOR.
- 2. SHOULD DUST BECOME A PROBLEM AT THE SITE, THE CONTRACTOR SHALL PROVIDE WATERING OR OTHER METHOD OF DUST CONTROL ACCEPTABLE TO THE WASHTENAW COUNTY WATER RESOURCES COMMISSIONER.
- 3. TEMPORARY STONE ACCESS DRIVE:
- 3.1. CRUSHED LIMESTONE BASE SHALL BE PLACED ON A GEOTEXTILE FILTER CLOTH OR APPROVED ALTERNATIVE ADDITIONAL LAYERS OF STONE OR CRUSHED LIMESTONE
- BASE SHALL BE ADDED IN LAYERS AND COMPACTED. 3.3. STEPS SHALL BE TAKEN TO REPAIR IF RUTS OR POOLING WATER APPEAR.
- 4.1. BUILT UP SEDIMENT SHALL BE REMOVED WHEN SEDIMENT ACCUMULATES TO 1/3 TO 1/2 OF THE HEIGHT OF THE SILT
- 4.2. IF SILT FABRIC DECOMPOSES OR BECOMES INEFFECTIVE PRIOR TO THE END OF THE EXPECTED USEABLE LIFE, AND THE BARRIER IS STILL REQUIRED, THE FABRIC SHALL BE PROMPTLY REPLACED.
- INLET FILTERS SHALL BE INSPECTED WEEKLY UNDER NORMAL CONDITIONS, WITHIN 24 HOURS OF RAINFALL AND DAILY DURING PROLONGED RAIN.
- PROMPTLY. 5.3. IF FABRIC DECOMPOSES OR BECOMES INEFFECTIVE PRIOR TO THE END OF THE EXPECTED USEABLE LIFE AND THE

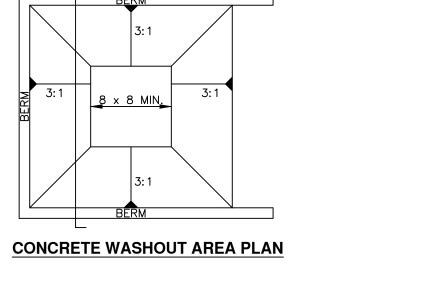
5.2. BUILT-UP SEDIMENT AND DEBRIS SHALL BE REMOVED BARRIER IS STILL REQUIRED, INLET FILTER SHALL BE

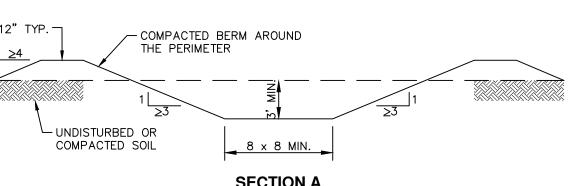
EXPANSION -RESTRAINT (NYLON ROPE, 2 FLAT WASHERS)

GEOTEXTILE SILT FENCE

DUMP STRAP 1" REBAR FOR BAG REMOVAL FROM INLET

> **INLET FILTER** NOT TO SCALE



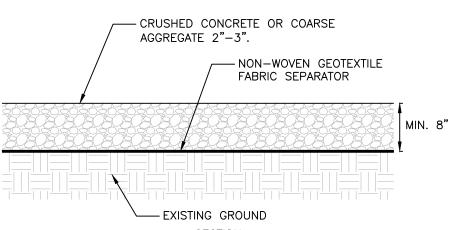


CWA INSTALLATION NOTES

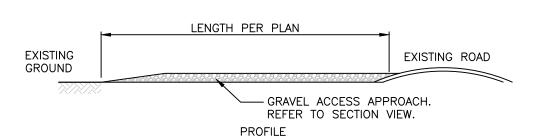
- 1. SEE PLAN VIEW FOR: INSTALLATION LOCATION.
- 2. DO NOT LOCATE ANY UNLINED CWA WITHIN 400' OF ANY NATURAL DRAINAGE PATHWAY OR WATERBODY. DO NOT LOCATE WITHIN 1,000' OF ANY WELLS OR DRINKING WATER SOURCES. IF SITE CONSTRAINTS MAKE THIS INFEASIBLE, OR IF HIGHLY PERMEABLE SOILS EXIST ON SITE, THE CWA MUST BE INSTALLED WITH AN IMPERMEABLE LINER (16 MIL MIN. THICKNESS) OR SURFACE STORAGE ALTERNATIVES USING PREFABRICATED CONCRETE WASHOUT DEVICES OR A LINED ABOVE GROUND STORAGE AREA SHOULD BE
- 3. THE CWA SHALL BE INSTALLED PRIOR TO CONCRETE PLACEMENT ON SITE.
- 4. CWA SHALL INCLUDE A FLAT SUBSURFACE PIT THAT IS AT LEAST 8' BY 8'. SLOPES LEADING OUT OF THE SUBSURFACE PIT SHALL BE 3:1 OR FLATTER. THE PIT SHALL BE AT LEAST 3' DEEP.
- 5. BERM SURROUNDING SIDES AND BACK OF THE CWA SHALL HAVE A MINIMUM HEIGHT OF 1'.
- 6. SIGNS SHALL BE PLACED AT THE CONSTRUCTION ENTRANCE, AT THE CWA, AND ELSEWHERE AS NECESSARY TO CLEARLY INDICATE THE LOCATION OF THE CWA TO OPERATORS OF CONCRETE TRUCKS
- 7. USE EXCAVATED MATERIAL FOR PERIMETER BERM CONSTRUCTION.

CONCRETE WASHOUT AREA

NOT TO SCALE



INSTALL AT CONSTRUCTION VEHICLE ENTRANCE/EXIT AS SPECIFIED PER PLAN. MINIMUM 15' WIDE X 50' LONG.



TRAP OR SEDIMENT BASIN.

- 1. THE ENTRANCES SHALL BE MAINTAINED IN A CONDITION THAT WILL PREVENT TRACKING OR FLOWING OF SEDIMENT ONTO PUBLIC RIGHT OF WAYS. THIS MAY REQUIRE TOP DRESSING, REPAIR AND/OR CLEANOUT OF ANY MEASURES USED TO TRAP SEDIMENT.
- SUPPLY WATER TO WASH TIRES IF NECESSARY, WHEN NECESSARY, WHEELS SHALL BE CLEANED PRIOR TO ENTRANCE ONTO PUBLIC RIGHT OF WAY. WHEN WASHING IS REQUIRED, IT SHALL BE DONE ON AN AREA STABILIZED

WITH CRUSHED AGGREGATE THAT DRAINS INTO AN APPROVED SEDIMENT

TEMPORARY CONSTRUCTION ENTRANCE

NOT TO SCALE



THE LOCATIONS OF EXISTING JNDERGROUND UTILITIES ARE SHOWN N AN APPROXIMATE WAY ONLY AND HAVE NOT BEEN INDEPENDENTLY VERIFIED. THE CONTRACTOR SHALL DETERMINE THE EXACT LOCATION OF ALL EXISTING UTILITIES BEFORE COMMENCING WORK, AND AGREES T BE FULLY RESPONSIBLE FOR ANY AND ALL DAMAGES WHICH MIGHT BE

OCCASIONED BY THE CONTRACTOR'S

FAILURE TO EXACTLY LOCATE AND

PRESERVE ANY AND ALL

UNDERGROUND UTILITIES. CONSTRUCTION SITE SAFETY IS THE RESPONSIBILITY OF THE CONTRACTOR. NEITHER THE OWNER NOR THE ENGINEER SHALL BE EXPECTED TO ASSUME ANY RESPONSIBILITY FOR SAFETY OF THI WORK, OF PERSONS ENGAGED IN THI WORK, OF ANY NEARBY

STRUCTURES, OR OF ANY OTHER

PERSONS.

08.27.2020 SITE PLAN SUBMISSION 09.29.2020
REVISED PER SITE PLAN REVIEW #1

10.22.2020
REVISED PER SITE PLAN REVIEW #2 12.07.20
REVISED PER SITE PLAN REVIEW #3



1051-19-9457 08/26/2020 Checked by Drawn by Crew/Boo CB | CB | KMW

AR BROUWER COMPANY

LEWIS JEWELERS

300 S. MAPLE ROAD ANN ARBOR, MICHIGAN 48103

ANN ARBOR WASHTENAW

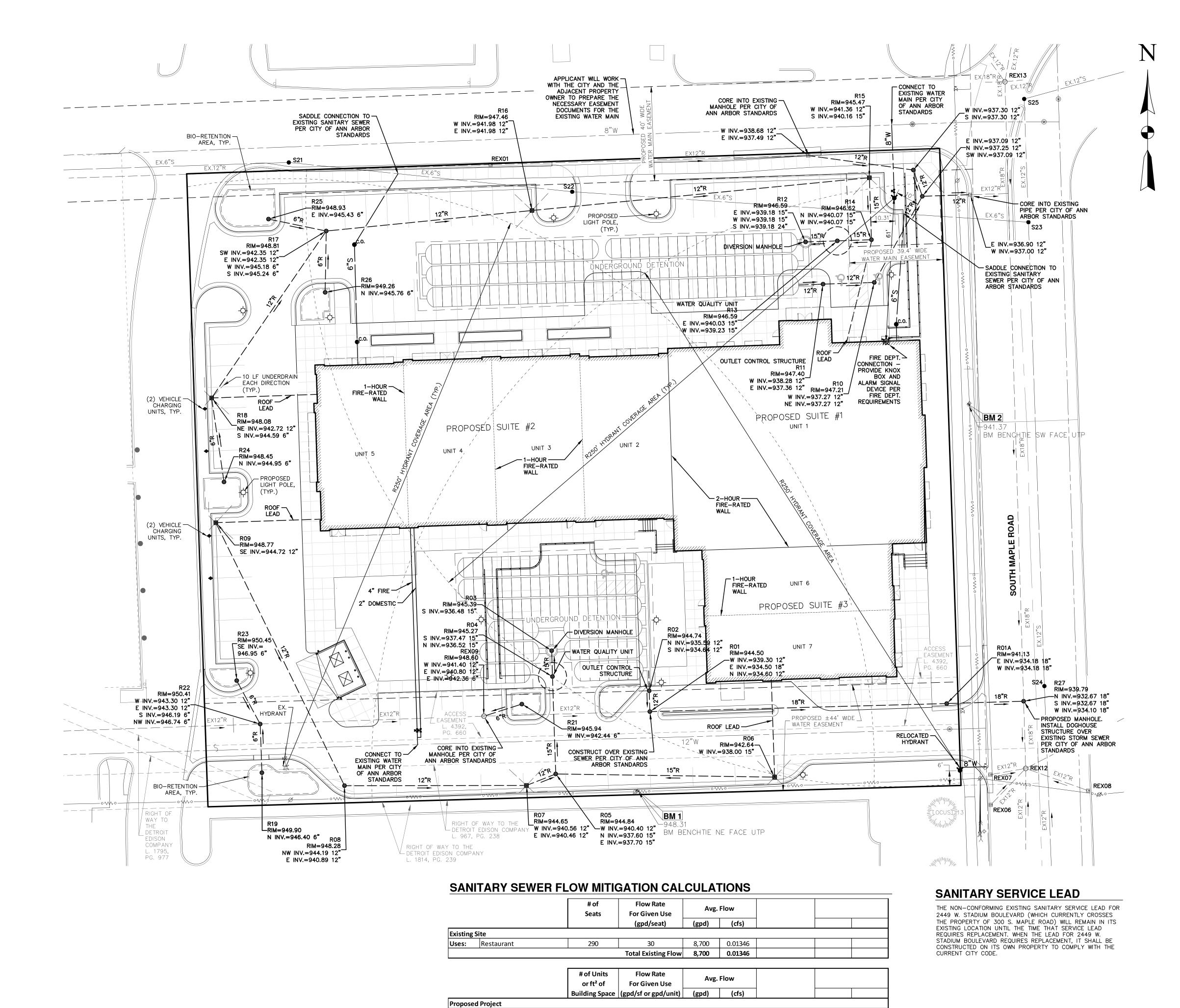
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SITE PLAN SOIL EROSION CONTOL

NOTES AND DETAILS

Sheet Number

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19,982

NOTE: Net reduction of sanitary flow. No sanitary sewer flow reuction is required on this project.

Uses: Retail Store

Uses: Restaurant

0.00093

0.00116

599 750

Total Proposed Flow 1,349 0.00209 Net Change In Flow 7350.54 0.0114

Design Basis: Use the City of Ann Arbor "Table A" and "Michigan Criteria for Subsurface Sewage Disposal" Michigan Department of Public Health April 1994 & the

LECENID

LEGEND	
× 656.5	EX. SPOT ELEVATION
	EX. CONTOUR EX. WETLAND LIMITS
	EY AD IACENT DEODERTY LINE
	EX. SECTION LINE EX. EASEMENT LINE
	EX. FENCE
	EX. GRAVEL EX. & DITCH
0 0	EX. GUARDRAIL
	EX. TREELINE EX. TREE (CONIFEROUS)
	EX. TREE (DECIDUOUS)
O MB	EX. SIGN
\circ_{F}	EX. MAILBOX FOUND IRON PIPE
O F-RR	FOUND IRON ROD
^O F−NAIL ⊚	FOUND PK NAIL FOUND CONC. MONUMENT
© S O S – NAIL	SET IRON ROD
O _S -NAIL	SET MAG NAIL
\	EX. SECTION CORNER
· — — — — — — — — — — — — — — — — — — —	SOIL BORING EX. WATER MAIN
W	EX. WATER VALVE
<u>@</u>	EX. HYDRANT EX. WATER MANHOLE
<u> </u>	EX. WELL
$\boxed{\mathbb{W}}$	EX. WATER METER
	EX. STORM SEWER EX. STORM INLET/CATCH BASIN
<u></u>	EX. STORM MANHOLE
	EX. STORM END SECTION EX. SANITARY SEWER
\$	EX. SANITARY MANHOLE
	EX. UNDERGROUND GAS
	EX. GAS VALVE EX. OVERHEAD ELECTRIC
	EX. UNDERGROUND ELECTRIC
	EX. UNDERGROUND CABLE EX. TELEPHONE MANHOLE
Ē	EX. ELECTRIC MANHOLE
E	EX. ELECTRIC METER EX. GAS METER
© E G -∳	EX. LIGHT POLE
150	EX. TRAFFIC SIGNAL BOX EX. UNIDENTIFIED MANHOLE
_ (i) Ø	EX. TRAFFIC SIGNAL POLE
Ø	EX. UTILITY POLE
— × 656.5	EX. GUY WIRE PR. SPOT ELEVATION
653	PR. CONTOUR
	PR. DRAINAGE ARROW PR. EASEMENT LINE
	PR. SETBACK LINE
	PR. CURB/PAVEMENT
	PR. FENCE PR. SIDEWALK RAMP
	PR. ASPHALT PAVEMENT
Δ 4 4	PR. CONCRETE PR. GUARDRAIL
0 0	PR. SIGN
	PR. WATER MAIN
	PR. WATER VALVE PR. HYDRANT
Ø	PR. WATER VALVE IN MANHOLE
	PR. WELL PR. REDUCER
	PR. STORM SEWER
	PR. STORM INLET/CATCH BASIN
	PR. STORM MANHOLE PR. STORM END SECTION
	PR. SANITARY SEWER
● • C.O.	PR. SANITARY MANHOLE PR. CLEANOUT
○ 1	PR. UTILITY CROSSING LOCATION
	PR. UNDERGROUND GAS
${}^{C}_{\!$	PR. GAS VALVE

BENCHMARKS:

BM #1: BENCH TIE IN NORTHEAST FACE OF UTILITY POLE ELEVATION: 948.31' (NAVD88) BM #2: BENCH TIE IN SOUTHWEST FACE OF UTILITY POLE

UTILITY NOTES:

ELEVATION: 941.37' (NAVD88)

1. PROPOSED ROOF LEADS SHALL BE SDR 23.5 PVC UNLESS OTHERWISE INDICATED AND/OR UNLESS OTHERWISE APPROVED BY THE AUTHORITY HAVING JURISDICTION.

PR. LIGHT POLE

- 2. ALL EXISTING UNDERDRAINS FROM THE BUILDING SHALL BE DISCONNECTED.
- 3. THE STORM SYSTEM WILL BE INSPECTED, AND WHEN NEEDED, CLEANED/SERVICED TO ENSURE THE SYSTEM IS 100% FUNCTIONAL
- 4. HYDRANTS SHALL BE IN SERVICE AND UNOBSTRUCTED DURING CONSTRUCTION.
- 5. "1-HOUR" RATED FIREWALLS SHALL BE CONSTRUCTED BETWEEN EACH TENANT'S UNIT AND "2-HOUR" RATED FIREWALLS SHALL BE CONSTRUCTED BETWEEN EACH SUITE. SEE PLAN FOR CALLOUTS.
- 6. THE PUBLIC WATER MAIN EASEMENT SHALL BE CLEAR OF OBSTRUCTIONS SUCH AS BUILDINGS, RETAINING WALL, DUMPSTER ENCLOSURES, OR ANY OTHER OBJECT WITH A FOUNDATION OR
- 7. THE EXISTING LIGHT POLES LOCATED WITHIN THE PROPOSED WATER MAIN EASEMENT WILL BE THE RESPONSIBILITY OF THE PROPERTY OWNER IN THE EVENT OF UTILITY MAIN MAINTENANCE, REPAIR OR REPLACEMENT.
- 8. THE SANITARY SEWER LEADS FOR 300 S. MAPLE ROAD AND 2449 W. STADIUM BOULEVARD ARE A SHARED SERVICE LEAD, WHICH IS NOT PERMITTED UNDER CURRENT CITY CODE. ADDITIONALLY, WE NOTE THAT THE EXISTING SANITARY SERVICE LEAD FOR THE ADJACENT PARCEL, 2449 W. STADIUM BOULEVARD, CROSSES THE SUBJECT SITE PLAN PARCEL (300 S. MAPLE ROAD). UTILITY SERVICE LEADS SERVING A PARCEL MUST CONNECT TO THE PUBLIC UTILITY FRONTING THAT PARCEL AND MAY NOT CROSS OVER OR IN FRONT OF ANOTHER PARCEL. THIS EXISTING, NON-CONFORMING SITUATION WILL BE ALLOWED TO REMAIN UNTIL THE SERVICE LEAD TO 2449 W. STADIUM BOULEVARD SERVICE LEAD REQUIRES REPLACEMENT, AT WHICH TIME IT MUST BE CONSTRUCTED ON ITS OWN PROPERTY
- 9. NO BOOSTER PUMPS ARE PROPOSED FOR THIS PROJECT.



THE LOCATIONS OF EXISTING JNDERGROUND UTILITIES ARE SHOWN N AN APPROXIMATE WAY ONLY AND HAVE NOT BEEN INDEPENDENTLY VERIFIED. THE CONTRACTOR SHALL DETERMINE THE EXACT LOCATION O ALL EXISTING UTILITIES BEFORE COMMENCING WORK, AND AGREES T BE FULLY RESPONSIBLE FOR ANY AND ALL DAMAGES WHICH MIGHT BE OCCASIONED BY THE CONTRACTOR'S FAILURE TO EXACTLY LOCATE AND PRESERVE ANY AND ALL UNDERGROUND UTILITIES.

CONSTRUCTION SITE SAFETY IS THE RESPONSIBILITY OF THE CONTRACTOR. NEITHER THE OWNER NOR THE ENGINEER SHALL BE EXPECTED TO ASSUME ANY RESPONSIBILITY FOR SAFETY OF THE WORK, OF PERSONS ENGAGED IN TH WORK, OF ANY NEARBY STRUCTURES, OR OF ANY OTHER PERSONS.

08.27.2020 SITE PLAN SUBMISSION

09.29.2020 A REVISED PER SITE PLAN REVIEW #1

10.22.2020 REVISED PER SITE PLAN REVIEW #2

12.07.20
REVISED PER SITE PLAN REVIEW #3

RELATIONSHIPS - REPUTATION - RESULTS 800.525.6016

1051-19-9457 08/26/2020 | Checked by | Drawn by | Crew/Book CB CB KMW

AR BROUWER COMPANY

LEWIS JEWELERS

te Address 300 S. MAPLE ROAD ANN ARBOR, MICHIGAN 48103

WASHTENAW ANN ARBOR 2S 5E 25

CHRISTOPHER SUTTON, PE --

SITE PLAN

UTILITY PLAN

Sheet Number

DRAINAGE NARRATIVE

THE DEVELOPER IS CONSTRUCTING A BUILDING WITH MULTIPLE SUITES FOR VARIED USE, INCLUDING A JEWELRY STORE ON 0.435 ACRES LOCATED ON THE WEST SIDE OF SOUTH MAPLE ROAD IN ANN ARBOR, WASHTENAW COUNTY, MICHIGAN. THE PROPOSED DEVELOPMENT INCLUDES THE CONSTRUCTION OF A THREE BUILDINGS ALONG WITH ASSOCIATED PARKING, UTILITIES AND STORM WATER MANAGEMENT

THE EXISTING SITE GENERALLY SLOPES TO THE SOUTH AND TO THE EAST. THE PROPOSED DRAINAGE FROM THE PROJECT AREA WILL BE COLLECTED BY AN ON-SITE STORM SEWER SYSTEM WHICH HAS BEEN DESIGNED TO CONVEY A 10-YEAR STORM. RUNOFF WILL BE TREATED BY A WATER QUALITY UNIT AND TWO UNDERGOUND DETENTION SYSTEMS ON BOTH THE NORTH AND SOUTH SIDES OF THE BUILDINGS. PRIOR TO DISCHARGING TO THE EXISTING MAIN IN SOUTH MAPLE ROAD. THE DISCHARGE FROM THE PROPOSED DETENTION SYSTEMS HAVE BEEN DESIGNED TO CONTROL THE RELEASE OF THE STORM IN ACCORDANCE WITH WASHTENAW COUNTY'S STANDARDS.



C-FACTOR CALCULATIONS

METRO CONSULTING ASSOCIATES

GROUND ELEV. INVERT ELEV.

Storm	Impervious	C-Factor	Pervious	C-Factor	Total Area	Total Area	Weighted
Structure	Area (sf)	C-I actor	Area (sf)	C-I actor	(sf)	(ac)	C-Factor
R01A	0	0.95	0	0.30	0	0.00	0.00
R01	0	0.95	0	0.30	0	0.00	0.00
R02	0	0.95	0	0.30	0	0.00	0.00
R03	0	0.95	0	0.30	0	0.00	0.00
R04	0	0.95	0	0.30	0	0.00	0.00
R05	0	0.95	0	0.30	0	0.00	0.00
R06	18,627	0.95	1,807	0.30	20,434	0.47	0.89
R07	7,747	0.95	3,516	0.30	11,263	0.26	0.75
R08	0	0.95	0	0.30	0	0.00	0.00
R09	3,218	0.95	371	0.30	3,589	0.08	0.88
R10	0	0.95	0	0.30	0	0.00	0.00
R11	0	0.95	0	0.30	0	0.00	0.00
R12	0	0.95	0	0.30	0	0.00	0.00
R13	0	0.95	0	0.30	0	0.00	0.00
R14	5,696	0.95	0	0.30	5,696	0.13	0.95
R15	10,525	0.95	219	0.30	10,744	0.25	0.94
R16	6,920	0.95	193	0.30	7,113	0.16	0.93
R17	0	0.95	0	0.30	0	0.00	0.00
R18	14,457	0.95	563	0.30	15,020	0.34	0.93
R19	580	0.95	1,239	0.30	1,819	0.04	0.51
R20	0	0.95	0	0.30	0	0.00	0.00
R21	86	0.95	218	0.30	304	0.01	0.48
R22	0	0.95	0	0.30	0	0.00	0.00
R23	0	0.95	201	0.30	201	0.00	0.30
R24	0	0.95	341	0.30	341	0.01	0.30
R25	0	0.95	450	0.30	450	0.01	0.30
R26	223	0.95	267	0.30	490	0.01	0.60
R27	0	0.95	0	0.30	0	0.00	0.00
R28	0	0.95	0	0.30	0	0.00	0.00
Total	68,079	0.95	9,385	0.30	77,464	1.74	0.87

Storm	Impervious	C-Factor	Pervious	C-Factor	Total Area	lotal Area	weighted
Structure	Area (sf)	C-i actoi	Area (sf)	C-i actor	(sf)	(ac)	C-Factor
R01A	0	0.95	0	0.30	0	0.00	0.00
R01	0	0.95	0	0.30	0	0.00	0.00
R02	0	0.95	0	0.30	0	0.00	0.00
R03	0	0.95	0	0.30	0	0.00	0.00
R04	0	0.95	0	0.30	0	0.00	0.00
R05	0	0.95	0	0.30	0	0.00	0.00
R06	18,627	0.95	1,807	0.30	20,434	0.47	0.89
R07	7,747	0.95	3,516	0.30	11,263	0.26	0.75
R08	0	0.95	0	0.30	0	0.00	0.00
R09	3,218	0.95	371	0.30	3,589	0.08	0.88
R10	0	0.95	0	0.30	0	0.00	0.00
R11	0	0.95	0	0.30	0	0.00	0.00
R12	0	0.95	0	0.30	0	0.00	0.00
R13	0	0.95	0	0.30	0	0.00	0.00
R14	5,696	0.95	0	0.30	5,696	0.13	0.95
R15	10,525	0.95	219	0.30	10,744	0.25	0.94
R16	6,920	0.95	193	0.30	7,113	0.16	0.93
R17	0	0.95	0	0.30	0	0.00	0.00
R18	14,457	0.95	563	0.30	15,020	0.34	0.93
R19	580	0.95	1,239	0.30	1,819	0.04	0.51
R20	0	0.95	0	0.30	0	0.00	0.00
R21	86	0.95	218	0.30	304	0.01	0.48
R22	0	0.95	0	0.30	0	0.00	0.00
R23	0	0.95	201	0.30	201	0.00	0.30
R24	0	0.95	341	0.30	341	0.01	0.30
R25	0	0.95	450	0.30	450	0.01	0.30
R26	223	0.95	267	0.30	490	0.01	0.60
R27	0	0.95	0	0.30	0	0.00	0.00
R28	0	0.95	0	0.30	0	0.00	0.00
Total	68,079	0.95	9,385	0.30	77,464	1.74	0.87



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08.27.2020 SITE PLAN SUBMISSION O9.29.2020
REVISED PER SITE PLAN REVIEW #1

10.22.2020
REVISED PER SITE PLAN REVIEW #2 12.07.20
REVISED PER SITE PLAN REVIEW #3

800.525.6016

1051-19-9457 08/26/2020

AR BROUWER COMPANY

LEWIS JEWELERS

300 S. MAPLE ROAD ANN ARBOR, MICHIGAN 48103

WASHTENAW ANN ARBOR Township Range Section SE 25

CHRISTOPHER SUTTON, PE--

SITE PLAN STORM WATER MANAGEMENT PLAN

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10-YEAR STORM SEWER DESIGN ON-SITE STORM SEWER CONVEYANCE SYSTEM DESIGN

Community: City of Ann Arbor Lewis Jewelers 9/24/2020 County: Washtenaw

Revision: 0 I = I = 151.8/(T+19.9)

Q manning = (1.486 A R2/3 S1/2)/nn concrete = 0.013 C = varies n plastic = 0.012T = 15 (min.)

MH	МН	MENT	С	AREA	AREA	TIME	(IN	C.F.S.	ITY OF	OF	OF	OF	BASED	2.5 FPS	HG	FLOW	OF	UPPER	LOWER	UPPER	LOWER	UPPER	LOWER	UPPER	LOWER
INPUT		ACRES		100%	100%	(MIN.)	PER	FLOW	SEWER	PIPE	LINE	PIPE	ON "Q"	GIVEN "D"	(%)	FULL	FLOW	END	END	END	END	END	END	END	END
		(A)		ACRES	ACRES		HOUR)		(C.F.S.)	(IN.)	(FT.)	(%)	(%)	(%)		(FT./	(MIN.)								1
				CA	SUM CA											SEC.)									1
R18	R17	0.34	0.93	0.32	0.32	15.00	4.35	1.39	2.34	12	86	0.43	0.15	0.30	0.30	3.0	0.5	943.42	943.15	948.08	948.81	942.72	942.35	4.66	5.66
R17	R16	0.00	0.00	0.00	0.32	15.50	4.29	1.39	2.34	12	88	0.43	0.15	0.30	0.30	3.0	0.5	943.04	942.78	948.81	947.46	942.35	941.98	5.77	4.68
R16	R15	0.16	0.93	0.15	0.47	16.00	4.23	1.99	2.34	12	144	0.43	0.31	0.30	0.31	3.0	0.8	942.61	942.16	947.46	945.47	941.98	941.36	4.85	3.31
R15	R14	0.25	0.94	0.23	0.70	16.80	4.14	2.91	3.65	15	26	0.32	0.20	0.23	0.23	3.0	0.1	942.13	942.07	945.47	946.62	941.16	941.07	3.34	4.55
R14	R13	0.13	0.95	0.12	0.83	16.90	4.13	3.41	3.65	15	14	0.32	0.28	0.23	0.28	3.0	0.1	942.07	942.03	946.62	946.59	941.07	941.03	4.55	4.56
R13	R12	0.00	0.00	0.00	0.83	17.00	4.11	3.41	3.65	15	13	0.32	0.28	0.23	0.28	3.0	0.1	940.22	940.18	946.59	946.59	939.23	939.18	6.37	6.41
R9	R8	0.08	0.88	0.07	0.07	15.00	4.35	0.32	2.34	12	124	0.43	0.01	0.30	0.30	3.0	0.7	945.37	944.99	948.77	948.27	944.72	944.19	3.40	3.28
R8	R7	0.00	0.00	0.00	0.07	15.70	4.26	0.32	2.34	12	77	0.43	0.01	0.30	0.30	3.0	0.4	941.59	941.36	948.27	944.66	940.89	940.56	6.68	3.30
R7	R5	0.26	0.75	0.19	0.27	16.10	4.22	1.12	2.34	12	13	0.43	0.10	0.30	0.30	3.0	0.1	941.24	941.20	944.66	944.83	940.46	940.40	3.42	3.63
R6	R5	0.47	0.89	0.42	0.42	15.00	4.35	1.82	3.65	15	93	0.32	0.08	0.23	0.23	3.0	0.5	938.91	938.70	942.29	944.83	938.00	937.70	3.38	6.13
R5	R4	0.00	0.00	0.00	0.68	15.50	4.29	2.94	3.65	15	41	0.32	0.21	0.23	0.23	3.0	0.2	938.56	938.47	944.83	945.28	937.60	937.47	6.27	6.81
R4	R3	0.00	0.00	0.00	0.68	15.70	4.26	2.94	3.65	15	11	0.32	0.21	0.23	0.23	3.0	0.1	937.51	937.48	945.28	945.40	936.52	936.48	7.77	7.92

EQUIV. TOTAL T I Q=CIA CAPAC- DIAM. LENGTH SLOPE MIN HG HG FOR ACTUAL VEL. TIME

Total Site Area= Total Site Area Excl	uding "Self-Crediting	g" BMPs =	39403 39403	<u>0.90</u> acr <u>0.90</u> acr		
		RATIONAL MET	HOD VARIABLES ₂	No. of Contract Contract		
				Runoff Coefficient,		
Cover Type	Soil Type	Area (sf)	Area (Ac)	С	C*A	
Grass/Lawn	B, 2% to 6%	1583	0.04	0.3	0.01	
Gravel	B, 2% to 6%	0	0.00	0.85	0.00	
Pavement/Roof	B, 2% to 6%	37820	0.87	0.95	0.82	
Water Surface	B, 2% to 6%		0.00	1	0.00	
				Total-∑C*A=	0.84	

	Total-∑C*A=									
	0.90									
	0.92									
ABLES₃										
		0 11 1								

Weighted CN - Σ CN*A/ Σ ac =

Area Total-∑ac =

0.87

Vbf-pre (cf)= 327.87

0.90 0.92

Pervious Cover Type	Soil Type	Area (sf)	Area (Ac)	Curve Number (CN)	CN*A
Grass/Lawn	B, 2% to 6%	1583	0.04	69	2.51
Gravel	B, 2% to 6%	0	0.00	85	0.00
Pavement/Roof	B, 2% to 6%		0.00	98	0.00
Water Surface B, 2% to 6%			0.00	98	0.00
			•	Total-∑CN*A=	2.51
				Area Total-∑ac =	0.04

NRCS VARIABLES₃							
Impervious Cover Curve Number							
Туре	Soil Type	Area (sf)	Area (Ac)	(CN)	CN*A		
Grass/Lawn	B, 2% to 6%		0.00	69	0.00		
Gravel	B, 2% to 6%		0.00	85	0.00		
Pavement/Roof	B, 2% to 6%	37820	0.87	98	85.09		
Water Surface	B, 2% to 6%	0	0.00	98	0.00		
	•	•		Total-∑CN*A=	85.09		

₁Use this area for the remainder of the runoff calculations Weighted CN - ∑CN*A/∑ac = 2Required for first flush calculations ₃Required for bankfull and 100-year runoff calculations

SUBSURFACE DETENTION BASIN STANDARD METHOD RUNOFF VOLUME CALCULATIONS

First Flush Runoff Calculations

A= Total Site Areas (ac) Excluding "Self-Crediting" BMPs from Worksheet 1 =
C= Weighted Runoff Coefficient from Worksheet 1 =

Vff(cf) = 3033.66Vff= (1")(1'/12")(43560sf/1ac)*A*C

SUBSURFACE DETENTION BASIN STANDARD METHOD RUNOFF VOLUME CALCULATIONS

Pre-development Bankfull Runoff Calculations (Vbf-pre)

A.	2 year/24 hour storm event (in)	P= 2.35
В.	The pre-develoment land cover will be Good Cover Woods or Meadow. Determine the associated soil hydrologic group for the entire site and choose the curve number	CN= 58
C.	S=1000/CN-10	S(in)= 7.24
D.	Q=(P-0.2S)^2/(P+0.8S)	Q(in)= 0.10
E.	Total Site Area (sf) excluding "Self-Crediting" BMPs	A(sf)= 39403

SUBSURFACE DETENTION BASIN STANDARD METHOD RUNOFF VOLUME CALCULATIONS

Vbf-pre = Q(1/12)Area

Development Bankfull Runoff Calculations (Vhf-ner-nost)

Pervious Cover Post-Development Bankfull Runoff Calculations (Vbf-per-post)						
A.	2 year/24 hour storm event (in)	P= 2.35				
В.	Pervious Cover CN From Worksheet 1	CN= 69				
C.	S=1000/CN-10	S(in)= 4.49				
D.	Q=(P-0.2S)^2/(P+0.8S)	Q(in)= 0.35				
E.	Pervious Cover Area from Worksheet 1	A(sf)= 1583				
F.	Vbf-per-post = Q(1/12)Area	Vbf-per-post (cf)= 46.75				

SUBSURFACE DETENTION BASIN STANDARD METHOD RUNOFF VOLUME CALCULATIONS

Impervious Cover Post-Development Bankfull Runoff Calculations (Vbf-imp-post)

Α.	2 year/24 hour storm event (in)	P= 2.35
В.	Impervious Cover CN From Worksheet 1	CN= 98
C.	S=1000/CN-10	S(in)= 0.20
D.	Q=(P-0.2S)^2/(P+0.8S)	Q(in)= 2.12
E.	Impervious Cover Area from Worksheet 1	A(sf)= 37820
F.	Vbf-imp-post = Q(1/12)Area	Vbf-imp-post (cf)= 6686.81

SUBSURFACE DETENTION BASIN STANDARD METHOD RUNOFF VOLUME CALCULATIONS

Pervious Cover Post-Development100-Year Storm Runoff Calculations (V100-per-post)

Α.	100-year storm event (in)	P= 5.11
В.	Pervious Cover CN From Worksheet 1	CN= 69
C.	S=1000/CN-10	S(in)= 4.49
D.	Q=(P-0.2S)^2/(P+0.8S)	Q(in)= 2.04
E.	Pervious Cover Area from Worksheet 1	A(sf)= 1583
F.	V100-per-post = Q(1/12)Area	V100-per-post (cf)= 268.80

SUBSURFACE DETENTION BASIN STANDARD METHOD RUNOFF VOLUME CALCULATIONS

Impervious Cover Post-Development100-Year Storm Runoff Calculations (V100-imp-post)

Α.	100-year storm event (in)	P= 5.11
В.	Impervious Cover CN From Worksheet 1	CN= 98
C.	S=1000/CN-10	S(in)= 0.20
D.	Q=(P-0.2S)^2/(P+0.8S)	Q(in)= 4.87
Ε.	Impervious Cover Area from Worksheet 1	A(sf)= 37820
F.	V100-imp-post = Q(1/12)Area	V100-imp-post (cf)= 15358.07

SUBSURFACE DETENTION BASIN STANDARD METHOD RUNOFF VOLUME CALCULATIONS

Determine Time of Concentration for Applicable Flow Types (Tc-hrs)

Flow Type	К	Change in Elevation (ft)	Length (ft)	Slope %	٧S	V=K*√S	Tc=L/(V*3600)
Sheet Flow*	0.48	Lievation (It)	Length (rt)	0.00	0.00	0.00	0.00
	4.0			0.00	2.00	0.00	0.00
Waterway	1.2			0.00	0.00	0.00	0.00
Waterway	1.2			0.00	0.00	0.00	0.00
Waterway	1.2			0.00	0.00	0.00	0.00
Waterway	1.2			0.00	0.00	0.00	0.00
Waterway	1.2			0.00	0.00	0.00	0.00
Waterway	1.2			0.00	0.00	0.00	0.00
Waterway	1.2			0.00	0.00	0.00	0.00
Waterway	1.2			0.00	0.00	0.00	0.00
Waterway	1.2			0.00	0.00	0.00	0.00
Waterway	1.2			0.00	0.00	0.00	0.00
Waterway	1.2			0.00	0.00	0.00	0.00
Waterway	1.2			0.00	0.00	0.00	0.00
Small Tributary	2.1			0.00	0.00	0.00	0.00
Small Tributary	2.1			0.00	0.00	0.00	0.00
Small Tributary	2.1			0.00	0.00	0.00	0.00
Small Tributary	2.1			0.00	0.00	0.00	0.00
Small Tributary	2.1			0.00	0.00	0.00	0.00
Small Tributary	2.1			0.00	0.00	0.00	0.00
Small Tributary	2.1			0.00	0.00	0.00	0.00
Small Tributary	2.1			0.00	0.00	0.00	0.00
Small Tributary	2.1			0.00	0.00	0.00	0.00
Small Tributary	2.1			0.00	0.00	0.00	0.00
Small Tributary	2.1			0.00	0.00	0.00	0.00
	let e	eet. Anything beyon	nd this is	777.5		ntration (Tc-hrs)=	

W9 SUBSURFACE DETENTION BASIN STANDARD METHOD RUNOFF VOLUME CALCULATIONS

Runoff Summary & Onsite Infiltration Requirement

Runoff Summary from Previous Worksheets		
First Flush Volume (Vff)	3,033.66 cf	
Pre-Development Bankfull Runoff Volume (Vbf-pre)	327.87 cf	
Pervious Cover Post-Development Bankfull Volume (Vbf-per-post)	46.75 cf	
Impervious Cover Post-Development Bankfull Volume (Vbf-imp-post)	6,686.81 cf	
Total BF Volume (Vbf-post)	6,733.56 cf	
Pervious Cover Post-Development 100-Year Volume (V100-per-post)	268.80 cf	
Impervious Cover Post-Development 100-Year Volume (V100-imp-post)	15,358.07 cf	
Total 100-Year Volume (V100-post)	15,626.88 cf	
Determine Onsite Infiltration Requirement		
Subtract the Per-Development Bankfull from the Post-Development Bankfull Volun	ne.	
Total Post-Development Bankfull Volume (Vbf-post)	6,733.56 cf	
Pre-Development Bankfull Runoff Volume (Vbf-pre)	327.87 cf	
Bankfull Volume Difference	6,405.69 cf	
Compare the Bankfull Volume Difference with the First Flush Volume. The greater Requirement.	of the two is the On-site Infiltratio	'n
Onsite Infiltration Requirement (Vinf)	6,405.69 cf	
	First Flush Volume (Vff) Pre-Development Bankfull Runoff Volume (Vbf-pre) Pervious Cover Post-Development Bankfull Volume (Vbf-per-post) Impervious Cover Post-Development Bankfull Volume (Vbf-imp-post) Total BF Volume (Vbf-post) Pervious Cover Post-Development 100-Year Volume (V100-per-post) Impervious Cover Post-Development 100-Year Volume (V100-imp-post) Total 100-Year Volume (V100-post) Determine Onsite Infiltration Requirement Subtract the Per-Development Bankfull from the Post-Development Bankfull Volume Total Post-Development Bankfull Volume (Vbf-post) Pre-Development Bankfull Runoff Volume (Vbf-pre) Bankfull Volume Difference Compare the Bankfull Volume Difference with the First Flush Volume. The greater Requirement.	First Flush Volume (Vff) Pre-Development Bankfull Runoff Volume (Vbf-pre) 327.87 cf Pervious Cover Post-Development Bankfull Volume (Vbf-per-post) 46.75 cf Impervious Cover Post-Development Bankfull Volume (Vbf-imp-post) 6,686.81 cf Total BF Volume (Vbf-post) 6,733.56 cf Pervious Cover Post-Development 100-Year Volume (V100-per-post) 1268.80 cf Impervious Cover Post-Development 100-Year Volume (V100-imp-post) 15,358.07 cf Total 100-Year Volume (V100-post) 15,626.88 cf Determine Onsite Infiltration Requirement Subtract the Per-Development Bankfull From the Post-Development Bankfull Volume. Total Post-Development Bankfull Volume (Vbf-post) 6,733.56 cf Pre-Development Bankfull Runoff Volume (Vbf-pre) 327.87 cf Bankfull Volume Difference 6,405.69 cf Compare the Bankfull Volume Difference with the First Flush Volume. The greater of the two is the On-site Infiltratio Requirement.

W10 SUBSURFACE DETENTION BASIN STANDARD METHOD RUNOFF VOLUME CALCULATIONS

Detention		
Α.	Peak of the Unit Hydrograph	
	Tc From Worksheet 8	Tc (min) = 0.25
	Qp=238.6*Tc^(-0.82)	Qp(cfs/in-mi^2) = 743.6
	The property of the second	100000000000000000000000000000000000000
В.	Total Site Area (ac) excluding "Self-Crediting" BMPs	Area (ac) = 0.90
Б.	Total Site Area (as) excitating Sen electring Sivil S	/ ii ca (de) = 0.50
C.	And the state of t	
	Q100-per from Worksheet 6	Q100-per (in) = 2.04
	Q100-imp from Worksheet 7	Q100-imp (in) = 4.87
	Q100 = Q100-per + Q100-imp	Q100 (in) = 6.91
D.	Peak Flow (PF) = (Qp*Q100*A)/640	PF (cfs) = 7.26
E.	1.016.01	3,20,7,346
	$\Delta = PF - 0.15*A$	Δ (cfs) = 7.13

W11 SUBSURFACE DETENTION BASIN STANDARD METHOD RUNOFF VOLUME CALCULATIONS

Determine Applicable BMPs and Associated Volume Credits

Vret = 2*V100

Proposed BMP ₁	Area (sf)	Storage Volume ₂ (cf)	Ave. Design Infiltration Rate (in/hr)	Infiltration Volume During Storm ₃ (cf)	Total Volume Reduction ₄ (cf)
Porous Pavement w/ Infiltration Bed					
Infiltration Basin	·				
Subsurface Infiltration Bed	·		·		
Infiltration Trench	·		,		
Rain Garden/Bioretention					
Dry Well	·				
Vegitated Filter Strip	,		,		
Green Roof	·		,		
	Tot	tal Volume Reduction	Credit by Proposed S	Structural BMPs (cf) =	0.00

Runoff Volume Infiltration Requirement (Vinf) from Worksheet 9 = 6,405.69 Total Volume Reduction Credit by Proposed Structural BMPs (cf) = -6,405.69

₁Complete checklist from Chapter VI for each Structural BMP type ₂Storage volume as defined in individual BMP write-ups

₃Approximated as the average design infiltration rate over 6 hours multiplied by the BMP area: Infiltration Rate x 6 hours x BMP Area x Unit Conversions = Infiltration Volume (cf) ₄Total Volume Reduction Credit is the sum of the Storage Volume and the Infiltration Volume During Storm

Lewis Jewelers - North LIG

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0.80

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1.09

1.16

1.23

1.30

1.36

1.42

1.47

1.53

1.57

1.62

1.67

1.71

1.75

1.79

1.83

1.86

1.90

1.93

1.96

2.00

2.03

2.05

2.08

2.11

2.13

2.16

2.18

2.21

2.23

2.25

2.27

2.29

2.31

2.33

2.34

2.36

2.38

2.39

2.41

2.42

2.43

2.44

2.46

2.47

2.48

2.49

2.50

2.51

2.51

2.53

Vret (cf) = 31,253.75

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145.56

152.44

158.90

165.01

170.82

176.34

181.60

186.62

191.42

196.01

200.40

204.65

208.71

212.61

216.37

219.98

223.45

226.80

230.02

233.13

236.10

238.99

241.77

244.44

247.02

249.50

251.87

254.17

256.37

258.48

260.51

262.45

264.31

266.09

267.79

269.41

270.96

272.43

273.82

275.14

276.39

277.57

278.68

279.74

280.73

281.65

283.02

50.71

oject:	Lewis Jewelers - North UG					
Chamber Mod	del -	MC-4500				
Units -		Imperial				
Number of Ch	nambers -	112	Г			
Number of En	nd Caps -	8				
Voids in the s	tone (porosity) -	25	9			
Base of STO	RAGE Elevation -	938.32	f			
Amount of Sto	one Above Chambers -	43	iı			
Amount of Sto	one Below Chambers -	0	iı			
Area of syster	m -	4878	s			

✓ Include Perimeter Stone in Calculations

sf Min. Area - 4353 sf min. area

for modeling p	rmTech requires a minimum of 9 inches of stone below the chambers. This table or modeling purposes only. Please see the engineer's drawings for the actual out of stone under the chambers.								
kormTe	ch MC-4500 C	umulative St	torage Volu	ımes					
Height of	Incremental Single	Incremental	Incremental	Incremental	Incremental	Incremental Ch,	Cumulative		
System	Chamber	Single End Cap	Chambers	End Cap	Stone	EC and Stone	System	Elevation	
(inches)	(cubic feet)	(cubic feet)	(cubic feet)	(cubic feet)	(cubic feet)	(cubic feet)	(cubic feet)	(feet)	
103	0.00	0.00	0.00	0.00	101.63	101.63	19628.18	946.90	
102	0.00	0.00	0.00	0.00	101.63	101.63	19526.56	946.82	
101	0.00	0.00	0.00	0.00	101.63	101.63	19424.93	946.74	
100	0.00	0.00	0.00	0.00	101.63	101.63	19323.31	946.65	
99	0.00	0.00	0.00	0.00	101.63	101.63	19221.68	946.57	
98	0.00	0.00	0.00	0.00	101.63	101.63	19120.06	946.49	
97	0.00	0.00	0.00	0.00	101.63	101.63	19018.43	946.40	
96	0.00	0.00	0.00	0.00	101.63	101.63	18916.81	946.32	
95	0.00	0.00	0.00	0.00	101.63	101.63	18815.18	946.24	
94	0.00	0.00	0.00	0.00	101.63	101.63	18713.56	946.15	
93	0.00	0.00	0.00	0.00	101.63	101.63	18611.93	946.07	
92	0.00	0.00	0.00	0.00	101.63	101.63	18510.31	945.99	
91	0.00	0.00	0.00	0.00	101.63	101.63	18408.68	945.90	
90	0.00	0.00	0.00	0.00	101.63	101.63	18307.06	945.82	
89	0.00	0.00	0.00	0.00	101.63	101.63	18205.43	945.74	
88	0.00	0.00	0.00	0.00	101.63	101.63	18103.81	945.65	
87	0.00	0.00	0.00	0.00	101.63	101.63	18002.18	945.57	
86	0.00	0.00	0.00	0.00	101.63	101.63	17900.56	945.49	
85	0.00	0.00	0.00	0.00	101.63	101.63	17798.93	945.40	
84	0.00	0.00	0.00	0.00	101.63	101.63	17697.31	945.32	
83	0.00	0.00	0.00	0.00	101.63	101.63	17595.68	945.24	
82	0.00	0.00	0.00	0.00	101.63	101.63	17494.06	945.15	
81	0.00	0.00	0.00	0.00	101.63	101.63	17392.43	945.07	

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3.15

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3.52

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4.04

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4.67

4.81

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88.77

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78.97

75.86

73.16

70.75

68.56

66.53

64.64

62.87

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59.63

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56.70

55.35

54.05

52.80

51.61

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49.37

48.32

47.31

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45.39

44.49

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42.77

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33.31

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119.44

124.57

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169.59

178.91

187.02

194.25

200.81

206.90

212.58

217.89

222.88

227.61

232.10

236.39

240.46

244.36

248.09

251.66

255.08

258.38

261.54

264.58

267.51

270.33

273.04

275.66

278.18

280.61

282.95

285.21

287.39

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293.47

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297.17

298.94

300.56

302.16

303.69

305.17

306.58

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310.45

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17290.81 944.99

17087.56 944.82

16782.68 944.57

16681.06 944.49

15359.93 943.40

15258.31 943.32

15153.24 943.24

15041.80 943.15

14926.18 943.07

14806.74 942.99

14682.17 942.90

14541.98 942.82

14383.80 942.74

14214.21 942.65

14035.29 942.57

13848.27 942.49

13654.02 942.40

13453.22 942.32

13246.31 942.24

13033.73 942.15

12815.84 942.07

12592.95 941.99

12365.34 941.90

12133.24 941.82

11896.85 941.74

11656.39 941.65

11412.03 941.57

11163.93 941.49

10912.27 941.40

10657.19 941.32

10398.81 941.24

10137.27 941.15

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9872.69 941.07

8786.15 940.74

8227.35 940.57

6790.79 940.15

6201.98 939.99

5904.80 939.90

5605.86 939.82

5003.14 939.65

4699.45 939.57

4394.28 939.49

3779.77 939.32

3470.55 939.24

3160.10 939.15

2848.49 939.07

2535.75 938.99

1907.18 938.82

1274.83 938.65

957.36 938.57

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16579.43 944.40 16477.81 944.32 944.24 16274.56 944.15 944.07 943.99 943.90 15868.06 943.82 RELATIONSHIPS - REPUTATION - RESULTS 15766.43 943.74 15664.81 943.65 800.525.6016 15563.18 943.57 15461.56 943.49

> 1051-19-9457 08/26/2020 | Checked by | Drawn by | Crew/Book CB | CB | KMW

AR BROUWER COMPANY

LEWIS JEWELERS

300 S. MAPLE ROAD ANN ARBOR, MICHIGAN 48103

WASHTENAW ANN ARBOR 2S 5E 25

CHRISTOPHER SUTTON, PE--

SITE PLAN **STORM WATER MANAGEMENT CALCULATIONS - NORTH**

rawing Scale Sheet Number

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EXPECTED TO ASSUME ANY RESPONSIBILITY FOR SAFETY OF THE WORK, OF PERSONS ENGAGED IN THI WORK, OF ANY NEARBY STRUCTURES, OR OF ANY OTHER PERSONS. 08.27.2020 SITE PLAN SUBMISSION 09.29.2020 REVISED PER SITE PLAN REVIEW #1 10.22.2020 REVISED PER SITE PLAN REVIEW #2 12.07.20
REVISED PER SITE PLAN REVIEW #3

Know what's **below**.

Call before you dig.

THE LOCATIONS OF EXISTING JNDERGROUND UTILITIES ARE SHOWN

IN AN APPROXIMATE WAY ONLY AND HAVE NOT BEEN INDEPENDENTLY VERIFIED. THE CONTRACTOR SHALL DETERMINE THE EXACT LOCATION OF ALL EXISTING UTILITIES BEFORE COMMENCING WORK, AND AGREES TO BE FULLY RESPONSIBLE FOR ANY AND ALL DAMAGES WHICH MIGHT BE OCCASIONED BY THE CONTRACTOR'S FAILURE TO EXACTLY LOCATE AND PRESERVE ANY AND ALL UNDERGROUND UTILITIES.

CONSTRUCTION SITE SAFETY IS THE RESPONSIBILITY OF THE CONTRACTOR. NEITHER THE OWNER NOR THE ENGINEER SHALL BE

Total Site Area Excl	uding "Self-Creditin	g" BMPs =	3493	39 sf, =	0.80
		RATIONAL MET	HOD VARIABLES ₂		
				Runoff Coefficient,	
Cover Type	Soil Type	Area (sf)	Area (Ac)	С	C*A
Grass/Lawn	B, 2% to 6%	6225	0.14	0.3	0.04
Gravel	B, 2% to 6%	0	0.00	0.85	0.00
Pavement/Roof	B, 2% to 6%	29677	0.68	0.95	0.65
Water Surface	B, 2% to 6%		0.00	1	0.00
				Total-∑C*A=	0.69

Area Total-∑ac = Weighted C - $\sum C*A/\sum ac =$

Area Total-∑ac =

Weighted CN - ∑CN*A/∑ac =

0.68

NRCS VARIABLES₃					
Pervious Cover Type	Soil Type	Area (sf)	Area (Ac)	Curve Number (CN)	CN*A
Grass/Lawn	B, 2% to 6%	6225	0.14	69	9.86
Gravel	B, 2% to 6%	0	0.00	85	0.00
Pavement/Roof	B, 2% to 6%		0.00	98	0.00
Water Surface	B, 2% to 6%		0.00	98	0.00
		·		Total-∑CN*A=	9.86

Area Total-∑ac = 0.14 Weighted CN - ∑CN*A/∑ac =

CN*A Soil Type (CN) Area (Ac) Grass/Lawn B, 2% to 6% 0.00 0.00 B, 2% to 6% 0.00 Pavement/Roof B, 2% to 6% Water Surface B, 2% to 6%

SUBSURFACE DETENTION BASIN STANDARD METHOD RUNOFF VOLUME CALCULATIONS

₁Use this area for the remainder of the runoff calculations

₃Required for bankfull and 100-year runoff calculations

First Flush Runoff Calculations

2Required for first flush calculations

A= Total Site Areas (ac) Excluding "Self-Crediting" BMPs from Worksheet 1 = C= Weighted Runoff Coefficient from Worksheet 1 =

Vff= (1")(1'/12")(43560sf/1ac)*A*C

Vff(cf) = 2437.86

Vbf-pre (cf)= 290.73

0.80

SUBSURFACE DETENTION BASIN STANDARD METHOD RUNOFF VOLUME CALCULATIONS

Pre-development Bankfull Runoff Calculations (Vbf-pre)

Α.	2 year/24 hour storm event (in)	P= 2.35
В.	The pre-develoment land cover will be Good Cover Woods or Meadow. Determine the associated soil hydrologic group for the entire site and choose the curve number	CN= 58
C.	S=1000/CN-10	S(in)= 7.24
D.	Q=(P-0.2S)^2/(P+0.8S)	Q(in)= 0.10

Total Site Area (sf) excluding "Self-Crediting" BMPs A(sf)= 34939

Vbf-pre = Q(1/12)Area

SUBSURFACE DETENTION BASIN STANDARD METHOD RUNOFF VOLUME CALCULATIONS

Pervious Co	ver Post-Development Bankfull Runoff Calculations (Vbf-	per-post)
A.	2 year/24 hour storm event (in)	P= 2.35
В.	Pervious Cover CN From Worksheet 1	CN= 69
C.	S=1000/CN-10	S(in)= 4.49
D.	Q=(P-0.2S)^2/(P+0.8S)	Q(in)= 0.35
E.	Pervious Cover Area from Worksheet 1	A(sf)= 6225
F.	Vbf-per-post = Q(1/12)Area	Vbf-per-post (cf)= 183.85

SUBSURFACE DETENTION BASIN STANDARD METHOD RUNOFF VOLUME CALCULATIONS

Impervious Cover Post-Development Bankfull Runoff Calculations (Vbf-imp-post)

Α.	2 year/24 hour storm event (in)	P= 2.35
В.	Impervious Cover CN From Worksheet 1	CN= 98
C.	S=1000/CN-10	S(in)= 0.20
D.	Q=(P-0.2S)^2/(P+0.8S)	Q(in)= 2.12
Ε.	Impervious Cover Area from Worksheet 1	A(sf)= 29677
F.	Vbf-imp-post = Q(1/12)Area	Vbf-imp-post (cf)= 5247.08

SUBSURFACE DETENTION BASIN STANDARD METHOD RUNOFF VOLUME CALCULATIONS

Pervious Cover Post-Development100-Year Storm Runoff Calculations (V100-per-post)

Α.	100-year storm event (in)	P= 5.11
В.	Pervious Cover CN From Worksheet 1	CN= 69
C.	S=1000/CN-10	S(in)= 4.49
D.	Q=(P-0.2S)^2/(P+0.8S)	Q(in)= 2.04
Ε.	Pervious Cover Area from Worksheet 1	A(sf)= 6225
F.	V100-per-post = Q(1/12)Area	V100-per-post (cf)= 1057.04

SUBSURFACE DETENTION BASIN STANDARD METHOD RUNOFF VOLUME CALCULATIONS

Impervious Cover Post-Development100-Year Storm Runoff Calculations (V100-imp-post)

Α.	100-year storm event (in)	P= 5.11
В.	Impervious Cover CN From Worksheet 1	CN= 98
C.	S=1000/CN-10	S(in)= 0.20
D.	Q=(P-0.2S)^2/(P+0.8S)	Q(in)= 4.87
E.	Impervious Cover Area from Worksheet 1	A(sf)= 29677
F.	V100-imp-post = Q(1/12)Area	V100-imp-post (cf)= 12051.34

SUBSURFACE DETENTION BASIN STANDARD METHOD RUNOFF VOLUME CALCULATIONS

considered waterway.

Determine Time of Concentration for Applicable Flow Types (Tc-hrs)

Flow Type	К	Change in Elevation (ft)	Length (ft)	Slope %	٧S	V=K*vS	Tc=L/(V*3600)
Sheet Flow*	0.48	Elevation (It)	Length (It)	0.00	0.00	0.00	0.00
Sileet Flow	0.46			0.00	0.00	0.00	0.00
Waterway	1.2			0.00	0.00	0.00	0.00
Waterway	1.2			0.00	0.00	0.00	0.00
Waterway	1.2			0.00	0.00	0.00	0.00
Waterway	1.2			0.00	0.00	0.00	0.00
Waterway	1.2			0.00	0.00	0.00	0.00
Waterway	1.2			0.00	0.00	0.00	0.00
Waterway	1.2			0.00	0.00	0.00	0.00
Waterway	1.2			0.00	0.00	0.00	0.00
Waterway	1.2			0.00	0.00	0.00	0.00
Waterway	1.2			0.00	0.00	0.00	0.00
Waterway	1.2			0.00	0.00	0.00	0.00
Waterway	1.2			0.00	0.00	0.00	0.00
Small Tributary	2.1			0.00	0.00	0.00	0.00
Small Tributary	2.1			0.00	0.00	0.00	0.00
Small Tributary	2.1			0.00	0.00	0.00	0.00
Small Tributary	2.1			0.00	0.00	0.00	0.00
Small Tributary	2.1			0.00	0.00	0.00	0.00
Small Tributary	2.1			0.00	0.00	0.00	0.00
Small Tributary	2.1			0.00	0.00	0.00	0.00
Small Tributary	2.1			0.00	0.00	0.00	0.00
Small Tributary	2.1			0.00	0.00	0.00	0.00
Small Tributary	2.1			0.00	0.00	0.00	0.00
Small Tributary	2.1			0.00	0.00	0.00	0.00
*Sheet flow cannot	t exceed 300 fe	et. Anything beyon	d this is	Tot	al Time of Concen	tration (Tc-hrs)=	0.25

SUBSURFACE DETENTION BASIN STANDARD METHOD RUNOFF VOLUME CALCULATIONS

Runoff Summary & Onsite Infiltration Requirement

A.	Runoff Summary from Previous Worksheets		
	First Flush Volume (Vff)	2,437.86 cf	
	Pre-Development Bankfull Runoff Volume (Vbf-pre)	290.73 cf	
	Pervious Cover Post-Development Bankfull Volume (Vbf-per-post)	183.85 cf	
	Impervious Cover Post-Development Bankfull Volume (Vbf-imp-post)	5,247.08 cf	
	Total BF Volume (Vbf-post)	5,430.93 cf	
	Pervious Cover Post-Development 100-Year Volume (V100-per-post)	1,057.04 cf	
	Impervious Cover Post-Development 100-Year Volume (V100-imp-post)	12,051.34 cf	
	Total 100-Year Volume (V100-post)	13,108.38 cf	
В.	Determine Onsite Infiltration Requirement		
	Subtract the Per-Development Bankfull from the Post-Development Bankfull Volument	me.	
	Total Post-Development Bankfull Volume (Vbf-post)	5,430.93 cf	
	Pre-Development Bankfull Runoff Volume (Vbf-pre)	290.73 cf	
	Bankfull Volume Difference	5,140.20 cf	
	Compare the Bankfull Volume Difference with the First Flush Volume. The greater Requirement.	r of the two is the On-site Infilt	ration

5,140.20 cf

Vret (cf) = 26,216.76

W10 SUBSURFACE DETENTION BASIN STANDARD METHOD RUNOFF VOLUME CALCULATIONS

Onsite Infiltration Requirement (Vinf)

Detention		
A.	Peak of the Unit Hydrograph	
	Tc From Worksheet 8 Qp=238.6*Tc^(-0.82)	Tc (min) = 0.25 Qp(cfs/in-mi^2) = 743.63
В.	Total Site Area (ac) excluding "Self-Crediting" BMPs	Area (ac) = 0.80
C.	Q100-per from Worksheet 6 Q100-imp from Worksheet 7 Q100 = Q100-per + Q100-imp	Q100-per (in) = 2.04 Q100-imp (in) = 4.87 Q100 (in) = 6.91
D.	Peak Flow (PF) = (Qp*Q100*A)/640	PF (cfs) = 6.44
E.	Δ = PF - 0.15*A	Δ (cfs) = 6.32
F.	Vdet = Required Detention*1.2 Vdet = $(\Delta/PF)*V100*1.2$	Vdet (cf) = 15,436.21

W11 SUBSURFACE DETENTION BASIN STANDARD METHOD RUNOFF VOLUME CALCULATIONS

Determine Applicable BMPs and Associated Volume Credits

Vret = 2*V100

		Storage Volume ₂	Ave. Design Infiltration Rate	Infiltration Volume	Total Volume
Proposed BMP ₁	Area (sf)	(cf)	(in/hr)	During Storm ₃ (cf)	Reduction ₄ (cf)
Porous Pavement w/ Infiltration Bed					
Infiltration Basin					
Subsurface Infiltration Bed			,		
Infiltration Trench					
Rain Garden/Bioretention	2280		0.5	570.00	570.00
Dry Well					
Vegitated Filter Strip					
Green Roof					
	To	tal Volume Reduction	Credit by Proposed S	tructural BMPs (cf) =	570.00

Runoff Volume Infiltration Requirement (Vinf) from Worksheet 9 = 5,140.20 Total Volume Reduction Credit by Proposed Structural BMPs (cf) = -4,570.20

1Complete checklist from Chapter VI for each Structural BMP type ₂Storage volume as defined in individual BMP write-ups

₃Approximated as the average design infiltration rate over 6 hours multiplied by the BMP area: Infiltration Rate x 6 hours x BMP Area x Unit Conversions = Infiltration Volume (cf) ₄Total Volume Reduction Credit is the sum of the Storage Volume and the Infiltration Volume During Storm

Chamber Model -Number of Chambers -Number of End Caps -Voids in the stone (porosity) -Base of STORAGE Elevation -Amount of Stone Above Chambers -Amount of Stone Below Chambers -Area of system -

Lewis Jewelers - South UG

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StormTech requires a minimum of 9 inches of stone below the chambers. This table

✓ Include Perimeter Stone in Calculations 4367 sf Min. Area - 3718 sf min. area

is for modeling purposes only. Please see the engineer's drawings for the actual amount of stone under the chambers. Stone EC and Stone System Elevation Single End Cap | Chambers Chamber End Cap System 16525.42 944.12 90.98 90.98 0.00 0.00 0.00 0.00 90.98 90.98 16434.44 944.04 16343.46 0.00 0.00 0.00 0.00 90.98 90.98 943.95 0.00 0.00 0.00 0.00 90.98 90.98 16252.48 943.87 0.00 90.98 16161.50 943.79 0.00 0.00 0.00 90.98 0.00 0.00 0.00 0.00 90.98 90.98 16070.52 943.70 0.00 0.00 0.00 0.00 90.98 90.98 15979.54 943.62 15888.56 943.54 0.00 0.00 0.00 0.00 90.98 90.98 0.00 0.00 0.00 0.00 90.98 90.98 15797.58 943.45 15706.60 943.37 0.00 90.98 0.00 0.00 0.00 90.98 0.00 0.00 0.00 0.00 90.98 90.98 15615.63 943.29 0.00 0.00 0.00 0.00 90.98 90.98 15524.65 943.20 0.00 0.00 0.00 0.00 90.98 90.98 15433.67 943.12 15342.69 943.04 0.00 0.00 0.00 90.98 90.98 0.00 0.00 0.00 0.00 90.98 90.98 15251.71 942.95 0.00 0.00 0.00 0.00 90.98 90.98 15160.73 942.87 0.00 0.00 0.00 0.00 90.98 90.98 15069.75 942.79 0.00 0.00 0.00 0.00 90.98 90.98 14978.77 942.70 0.00 0.00 0.00 0.00 90.98 90.98 14887.79 942.62 0.00 0.00 0.00 0.00 90.98 90.98 14796.81 942.54

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14250.94 942.04

14068.98 941.87

13978.00 941.79

13887.02 941.70

13796.04 941.62

13705.06 941.54

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1843.75 936.20

1320.70 936.04

794.56 935.87

530.44 935.79

265.66 935.70

936.29

936.12

935.95

2104.01

1582.63

1058.00

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942.12

941.95

941.45

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1.02

1.03

0.98

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207.29

208.18

209.01

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211.24

212.26

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Know what's **below.** Call before you dig.

THE LOCATIONS OF EXISTING

JNDERGROUND UTILITIES ARE SHOWN IN AN APPROXIMATE WAY ONLY AND HAVE NOT BEEN INDEPENDENTLY VERIFIED. THE CONTRACTOR SHALL DETERMINE THE EXACT LOCATION OF ALL EXISTING UTILITIES BEFORE COMMENCING WORK, AND AGREES TO BE FULLY RESPONSIBLE FOR ANY AND ALL DAMAGES WHICH MIGHT BE OCCASIONED BY THE CONTRACTOR'S FAILURE TO EXACTLY LOCATE AND PRESERVE ANY AND ALL UNDERGROUND UTILITIES.

CONSTRUCTION SITE SAFETY IS THE RESPONSIBILITY OF THE CONTRACTOR. NEITHER THE OWNER NOR THE ENGINEER SHALL BE EXPECTED TO ASSUME ANY RESPONSIBILITY FOR SAFETY OF THE WORK, OF PERSONS ENGAGED IN THI WORK, OF ANY NEARBY STRUCTURES, OR OF ANY OTHER PERSONS.

08.27.2020 SITE PLAN SUBMISSION 09.29.2020 \(\text{REVISED PER SITE PLAN REVIEW #1} \) 10.22.2020
REVISED PER SITE PLAN REVIEW #2

12.07.20
REVISED PER SITE PLAN REVIEW #3

RELATIONSHIPS - REPUTATION - RESULTS 800.525.6016

1051-19-9457 08/26/2020 | Checked by | Drawn by | Crew/Book CB CB KMW

AR BROUWER COMPANY

LEWIS JEWELERS

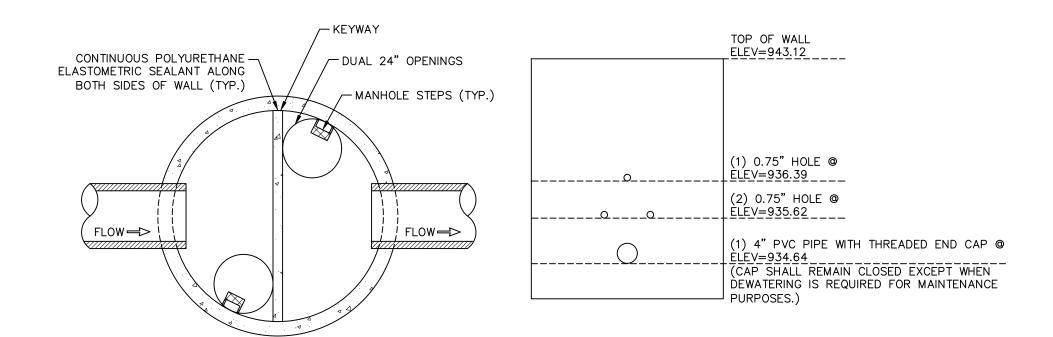
300 S. MAPLE ROAD ANN ARBOR, MICHIGAN 48103

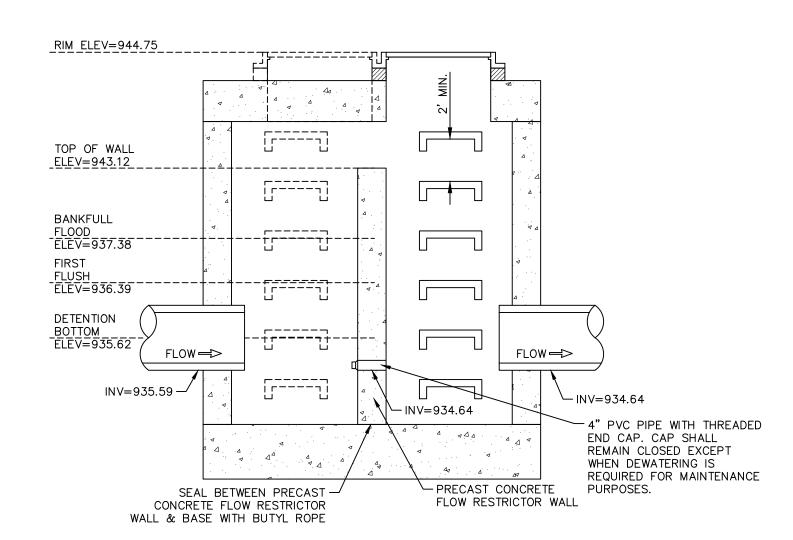
WASHTENAW ANN ARBOR 2S 5E 25

CHRISTOPHER SUTTON, PE--

SITE PLAN **STORM WATER MANAGEMENT CALCULATIONS - SOUTH**

rawing Scale Sheet Number





SECTION "A-A"

- 1. THIS TYPE OF FLOW RESTRICTOR STRUCTURE TO BE USED
- ONLY WITH UNDERGROUND DETENTION SYSTEMS..

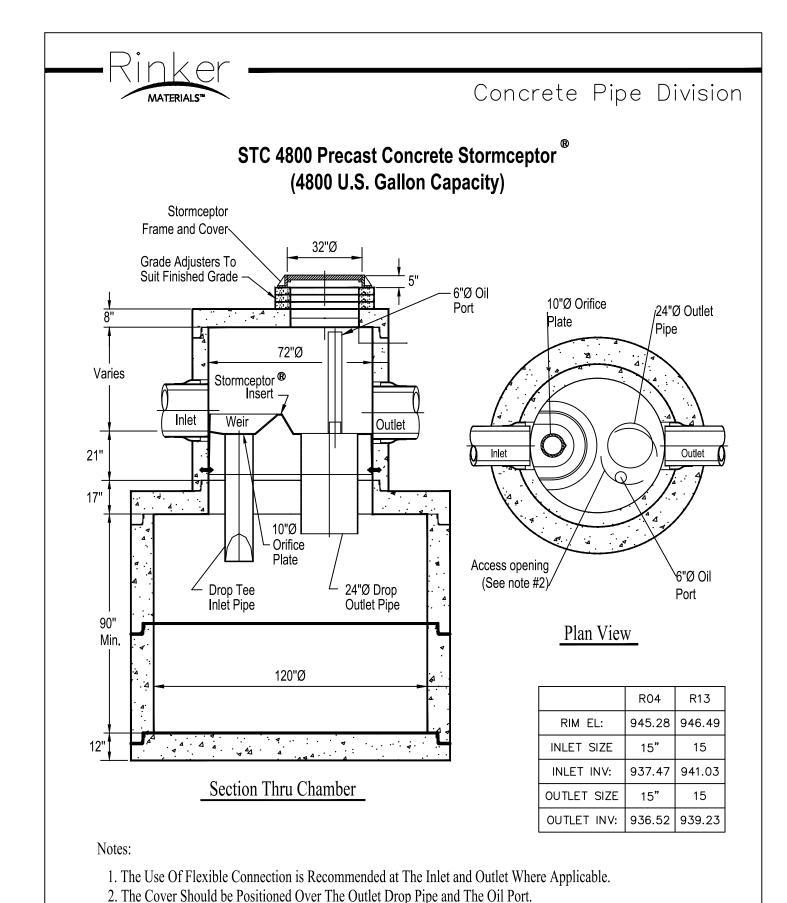
 2. THE PRECAST REINFORCED FLOW RESTRICTOR STRUCTURE
- SHALL BE MANUFACTURED PER ASTM C-478
 SPECIFICATIONS AND MEETING HS20 LOADING REQUIREMENTS
 3. CONTRACTOR TO PROVIDE SHOP DRAWING, INCLUDING

REINFORCEMENT, PRIOR TO CONSTRUCTION. OUTLET CONTROL STRUCTURE (R02)

Storage Elevations	$X_0 = Xbf =$	935.62 937.37	Xff = X100 =	936.39 942.99	
Outlet Control Structure					
1. outlet holes sizing - "first flush" runoff First Flush great he galacced in a gainsing great flush					
First Flush must be released in a mimimum of 24 hours		O#-	0.0200 -6		
Qff = Vff / 24 hrs / 3600 sec $\frac{1}{2} = \frac{2}{2} = 2$		Qff=	0.0280 cfs		
$h = 2/3 \times (Xff - Xo)$		h =	0.51 ft		
$A = Qff / .62 \times sqrt(2*32.2*h)$		A =	0.0079 sf		
The area of a 0.75" orifice			0.0031 sf	. 1	
Number of orifice holes		=		s at elev.	935.
Qact = $0.62 \text{ x #orif x Aorif x sqrt}(2x32.2xh) =$		Qact =	0.022 cfs		
Tff = Vff / Qact		Tff=	30.7 hrs		
2. outlet holes sizing - "Bankfull flood" discharge					
Bankfull must be detained between 36 to 48 hours					
$h = 2/3 \times (Xbf - Xo)$		h =	1.17 ft		
$Qbf = 0.62 \times \#orif \times Aorif \times sqrt(2x32.2xh) =$		Qbf=	0.033 cfs		
Tbf = Vbf / Qbf (with first flush holes only)		Tbf =	45.4 hrs		
Add holes at to decrease storage time					
The first flush volume will discharge in		Tff=	30.7 hrs		
The volume between the Bankfull elevation and the First Flush Elevation					
Vrem = Vbf - Vff		Vrem=	2,983 cf		
Trem = should discharge (target 48 hour discharge time) in		Trem=	17.3 hrs		
$Hff = 2/3 \times (Xbf - Xff) + (Xff - Xo)$		Hff=	1.43 ft		
Qff+bf = 0.62 x #orif x Aorif x sqrt(2x32.2xh)		Qff+bf=	0.036 cfs		
Vff+bf = Trem x Qff+bf		Vff+bf=	2,263 cf		
Vbf = Vrem - Vff+bf		Vbf=	720 cf		
Qbf = Vbf/Trem		Qbf=	0.012 cfs		
$Hbf = 2/3 \times (Xbf - Xff)$		Hbf=	0.66 ft		
Abf = Qbf / $0.62 \times \text{sqrt}(2*32.2*\text{Hbf})$		Abf=	0.0029 sf		
The area of a 0.75" orifice		1101	0.0031 sf		
Number of orifice holes		=	1 holes	s at elev	936.
Qact = $0.62 \times \text{#orif x Aorif x sqrt}(2x32.2xh) =$		Qact =	0.012 cfs	, at cic v	550.
Tbf = Tff + Vrem/(Qff + Qbf)*(3600/1)		Tbf=	47.7 hrs		
101 TH + VICHI7 (QH + Q01) (3000/1)		101	77.7 IIIS		
3. outlet holes sizing - "100-yr flood" discharge		۸	0.81 ac		
Area, $a = $ Qa = $0.15 \times A$		A = Qa =	0.81 ac 0.122 cfs		
	10 3700)				
Qff+Qbf = $0.62 \times \text{#fforif x Afforif x sgrt}(64.4 \times 100 - \text{Xo})) + 0.62 \times \text{#bforif x Abforif x sgrt}(64.4 \times 100 - \text{Xo})) + 0.62 \times \text{#bforif x Abforif x sgrt}(64.4 \times 100 - \text{Xo}))$	00 - XII))	Qff+Qbf = 0.100 = 0.000	0.122 cfs		
Q100 = Qa - (Qff + Qbf) $A100 = Q100 / (0.62 y cart/2*22.2 y (V100)) Vbf$		Q100 = 4.100 = 4.100	-0.001 cfs		
A100 = Q100 / (0.62 x sgrt(2*32.2 x(X100 - Xbf))		A100 =	0.0000 sf		
The area of a 0.75" orifice			0.0031 sf	4 . 1	
Number of orifice holes		=	0 holes		937.
Qff+ Qbf+ $0.62 \times \text{#orif x A} 100 \times \text{sgrt}(2*32.2*h100) < Qa$		TT 11	0.122 < Qa	ı	
Hall = $2/3 \times (X100 - Xbf) + (Xbf - Xo)$		Hall =	5.50 ft		
Qall = 0.62 x #fforif x Afforif x sqrt(2*32.2*Hall)		Qall =	0.072 cfs		
$Hbf = 2/3 \times (X100 - Xbf) + (Xbf - Xff)$		Hbf=	4.73 ft		
Qbf+100 = $0.62 \times \text{#bforif } x \text{ Abforif } x \text{ sqrt}(2*32.2*Hbf)$		Qbf+100 =	0.033 cfs		
$H100 = 2/3 \times (X100 - Xbf)$		H100 =	3.74 ft		
$Q100avg = 0.62 \times #100orif \times A100orif \times sqrt(2+32.2*H100)$		Q100avg =	0.000 cfs		
Vrem = V100 - Vbf		Vrem=	9,889 cf		

71.7 hrs

T100 =



WAYNE COUNTY NOTE:

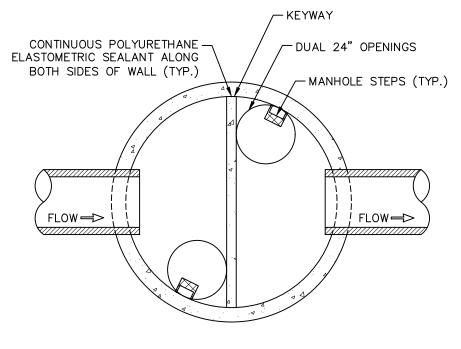
THE STORMCEPTOR STRUCTURE MUST BE FABRICATED AS PER SHOP DRAWINGS THAT HAVE BEEN APPROVED BY WAYNE COUNTY. THE MANUFACTURER MUST CONTACT WAYNE COUNTY TESTING OFFICE AT (734) 595-6504 x2015 AT LEAST 3 WORKING DAYS PRIOR TO FABRICATION TO SCHEDULE INSPECTION DURING FABRICATION.

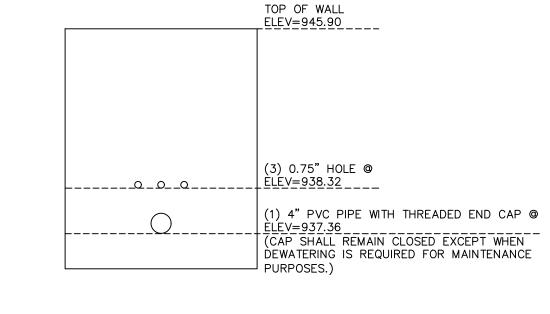
3. The Stormceptor System is protected by one or more of the following U.S. Patents: #4985148,

4. Contact a Concrete Pipe Division representative for further details not listed on this drawing.

#5498331, #5725760, #5753115, #5849181, #6068765, #6371690.

WATER QUALITY CONTROL STRUCTURE (RO4, R13)

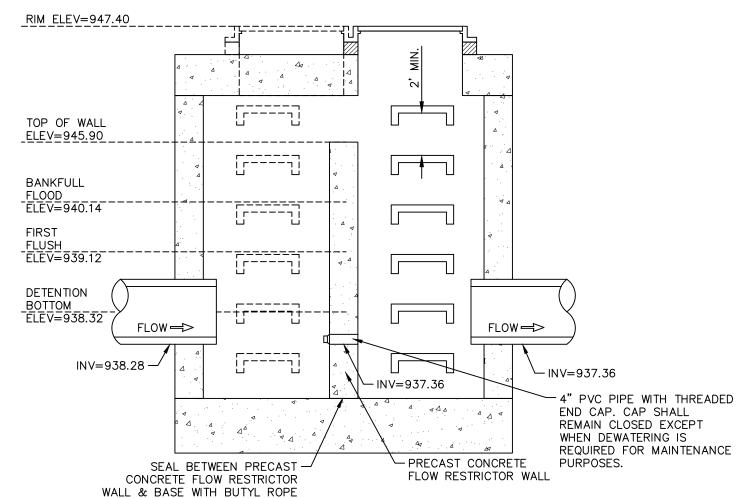




939.12

945.90

X100 =



SECTION "A-A"

- 1. THIS TYPE OF FLOW RESTRICTOR STRUCTURE TO BE USED
- ONLY WITH UNDERGROUND DETENTION SYSTEMS..

 2. THE PRECAST REINFORCED FLOW RESTRICTOR STRUCTURE
- SHALL BE MANUFACTURED PER ASTM C-478
 SPECIFICATIONS AND MEETING HS20 LOADING REQUIREMENTS
- 3. CONTRACTOR TO PROVIDE SHOP DRAWING, INCLUDING REINFORCEMENT, PRIOR TO CONSTRUCTION.

Storage Elevations

OUTLET CONTROL STRUCTURE (R11)

Xbf =

$\lambda 01$ –	940.14	A100 - 943.90	
Outlet Control Structure			
1. outlet holes sizing - "first flush" runoff			
First Flush must be released in a mimimum of 24 hours			
Qff = Vff / 24 hrs / 3600 sec	Qff=	0.035 cfs	
$h = 2/3 \times (Xff - Xo)$	h =	0.53 ft	
$A = Qff / .62 \times sqrt(2*32.2*h)$	A =	0.010 sf	
The area of a 0.75" orifice		0.0031 sf	
Number of orifice holes	=	3 holes at elev.	938.32
Qact = 0.62 x #orif x Aorif x sqrt(2x32.2xh) =	Qact =	0.033 cfs	
Tff = Vff/Qact	Tff=	25.2 hrs	
2. outlet holes sizing - "Bankfull flood" discharge			
Bankfull must be detained between 36 to 48 hours			
$h = 2/3 \times (Xbf - Xo)$	h =	1.21 ft	
$Qbf = 0.62 \times #orif \times Aorif \times sqrt(2x32.2xh) =$	Qbf=	0.050 cfs	
Tbf = Vbf/Qbf (with first flush holes only)	Tbf =	37.1 hrs	
3. outlet holes sizing - "100-yr flood" discharge			
Area, a =	A =	0.90 ac	
$Qa = 0.15 \times A$	Qa=	0.136 cfs	
Qff+Qbf = 0.62 x #fforif x Afforif x sqrt(64.4 (X100 - Xo)) + 0.62 x #bforif x Abforif x sqrt(64.4 (X100 - Xff))	Qff+Qbf=	0.126 cfs	
Q100 = Qa - (Qff + Qbf)	Q100 =	0.010 cfs	
A100 = Q100 / (0.62 x sqrt(2*32.2 x (X100 - Xbf))	A100 =	0.0008 sf	
The area of a 0.75" orifice		0.0031 sf	
Number of orifice holes	=	0 holes at elev	940.14
$Qff + Qbf + 0.62 \times \#orif \times A100 \times sqrt(2*32.2*h100) < Qa$		$0.126 \le Qa$	
$Hall = 2/3 \times (X100 - Xbf) + (Xbf - Xo)$	Ha 1 1 =	5.66 ft	
Qall = 0.62 x #fforif x Afforif x sqrt(2*32.2*Hall)	Qall=	0.109 cfs	
$Hbf = 2/3 \times (X100 - Xbf) + (Xbf - Xff)$	Hbf =	4.86 ft	
Qbf+100 = 0.62 x #bforif x Abforif x sqrt(2*32.2*Hbf)	Qbf+100 =	0.000 cfs	
$H100 = 2/3 \times (X100 - Xbf)$	H100 =	3.84 ft	
Q100avg = 0.62 x #100orif x A100orif x sqrt(2+32.2*H100)	Q100avg =	0.000 cfs	
Vrem = V100 - Vbf	Vrem=	11,668 cf	
T100 = Tbf + Vrem / (Qall + Qbf + 100 + Qall)	T100 =	66.9 hrs	



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08.27.2020
SITE PLAN SUBMISSION

09.29.2020
REVISED PER SITE PLAN REVIEW #1

10.22.2020
REVISED PER SITE PLAN REVIEW #2

12.07.20
REVISED PER SITE PLAN REVIEW #3

RELATIONSHIPS - REPUTATION - RESULTS 800.525.6016

Project Number | Date | 1051-19-9457 | 08/26/2020 | P.M. | Checked by | Drawn by | Crew/Book | CB | CB | KMW | .

AR BROUWER COMPANY

Ora inat

LEWIS JEWELERS

site Address

300 S. MAPLE ROAD
ANN ARBOR, MICHIGAN 48103

County Community
WASHTENAW ANN ARBOR

Township Range Section 2S 5E 25

Professional Name | Date | CHRISTOPHER SUTTON, PE--

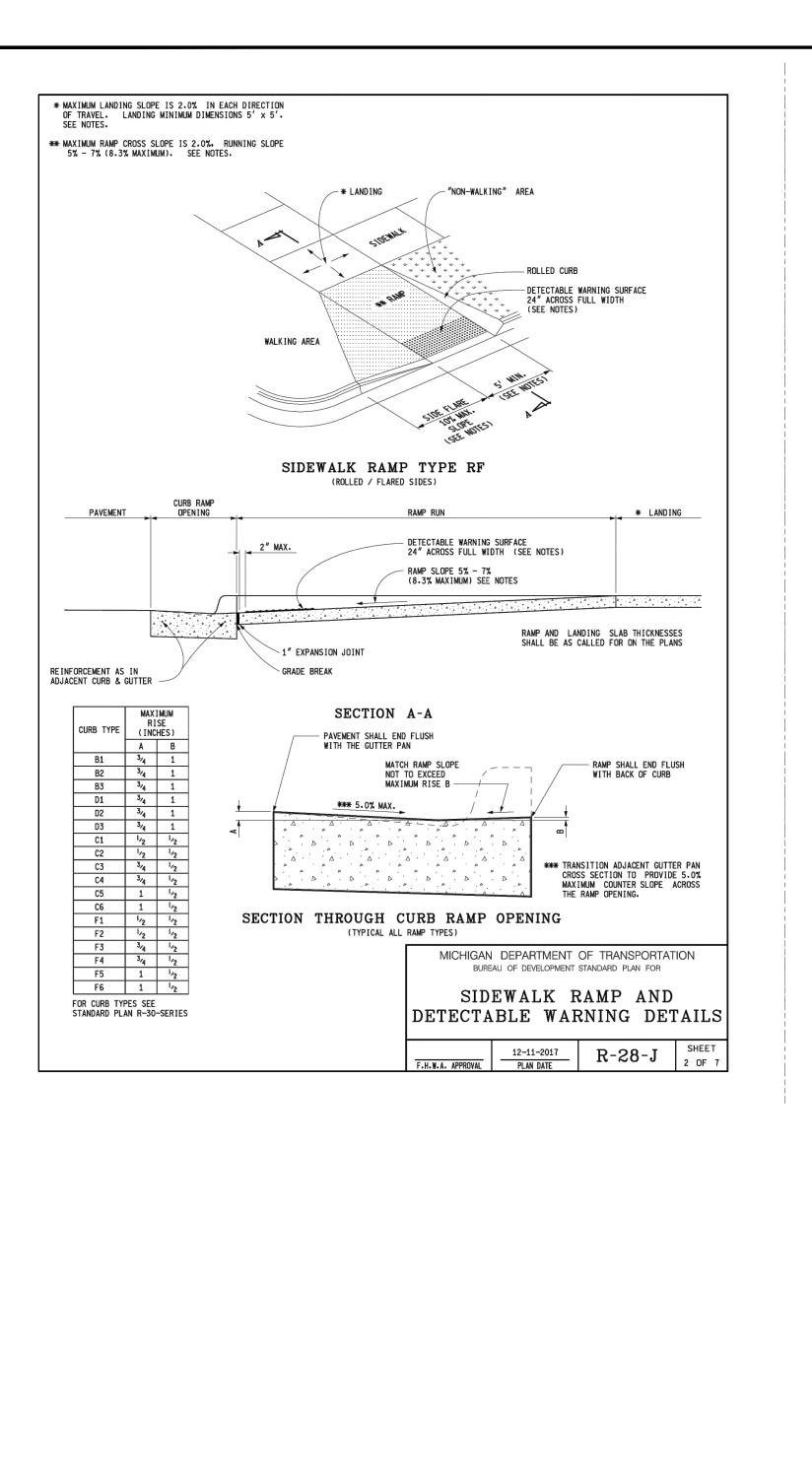
MANAGEMENT DETAILS

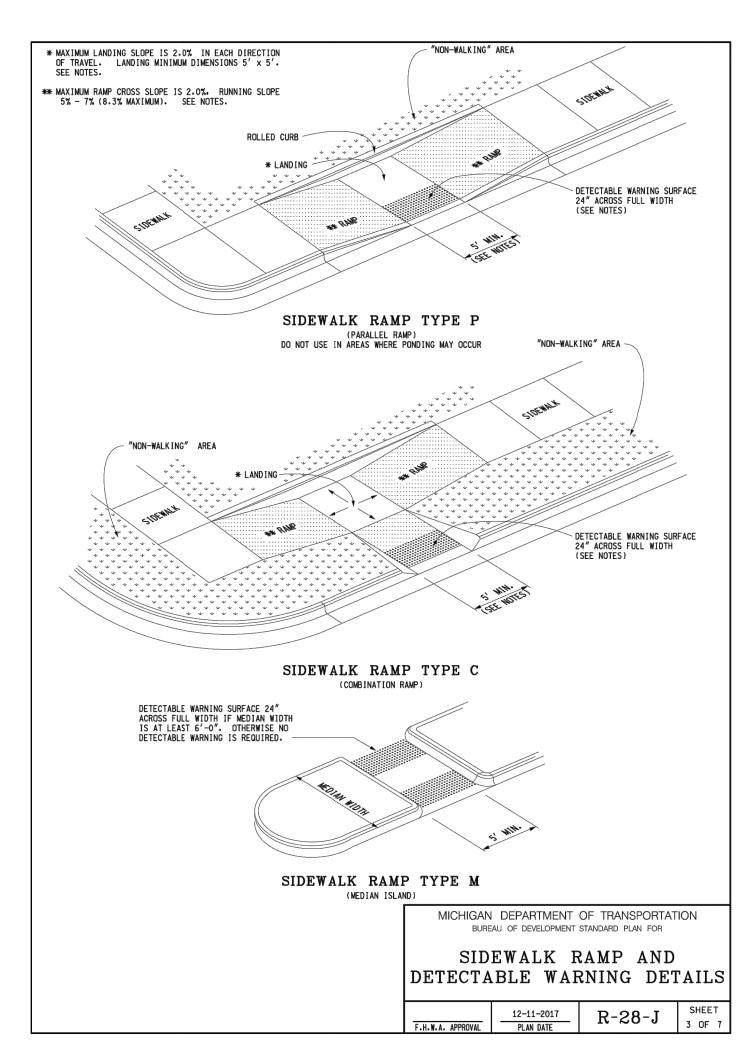
SITE PLAN
STORM WATER

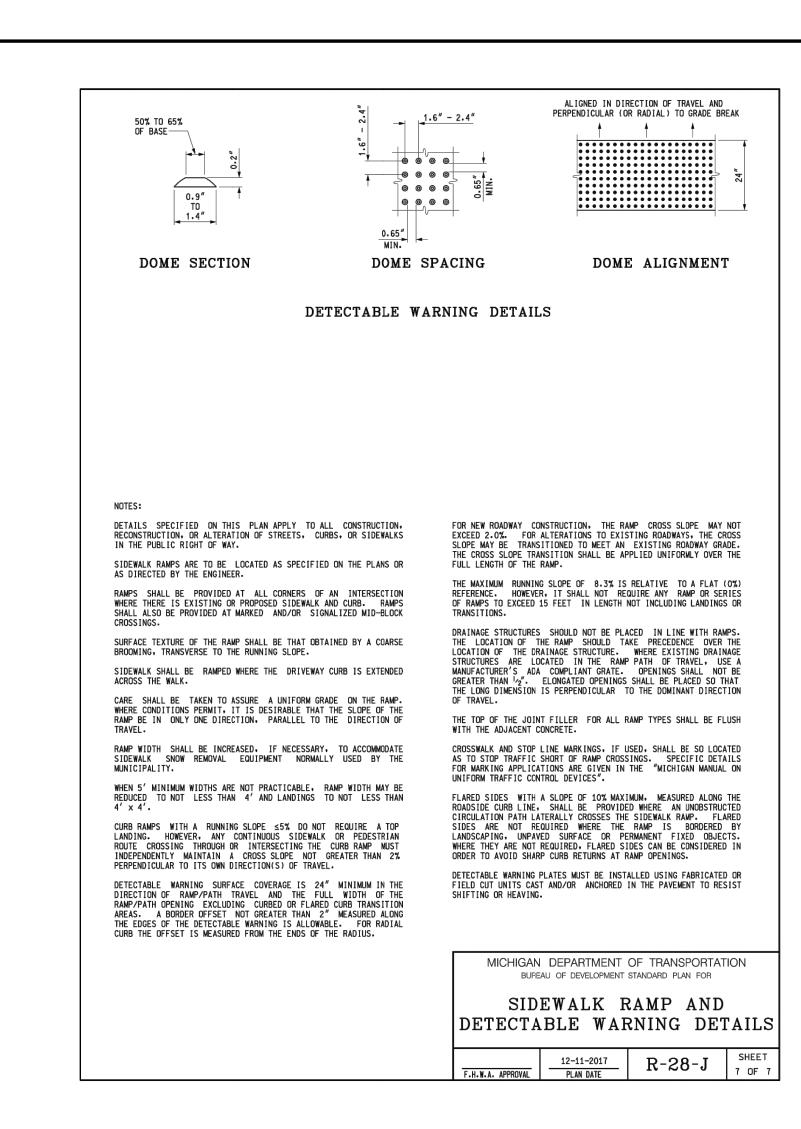
Drawing Scale

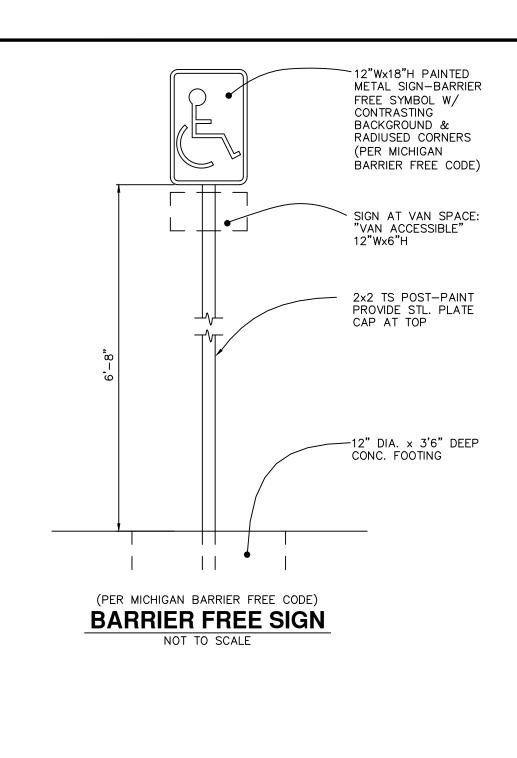
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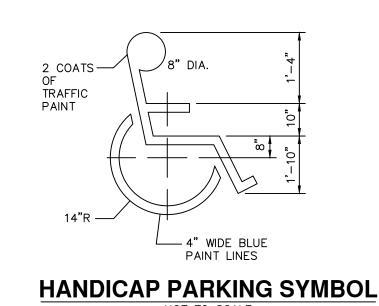
T100 = Tbf + Vrem / (Qall + Qbf + 100 + Qall)

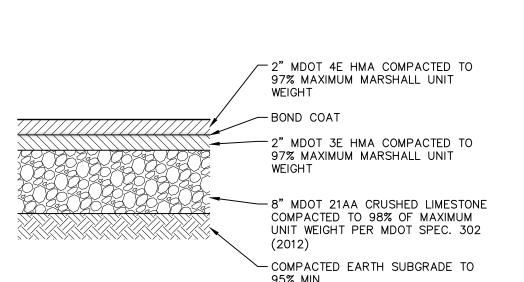






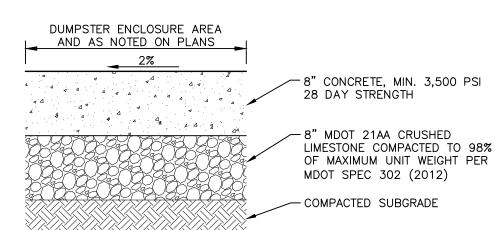




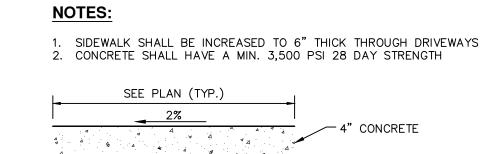




NOT TO SCALE

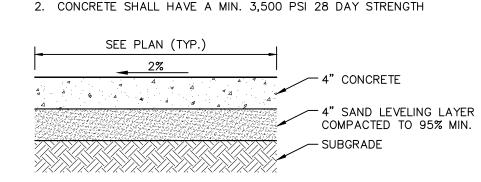


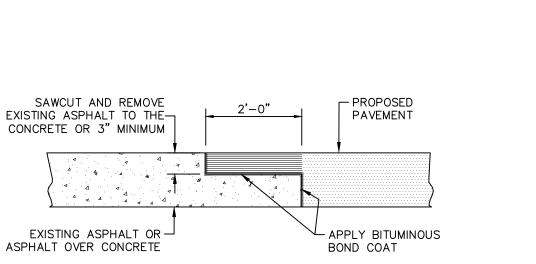
UTY CONCRETE PAVEMENT NOT TO SCALE





4	HE	AVY	/ C	4,
	OTES:			
1. 2.	SIDEWA CONCR	ALK SH ETE S	IALL HALI	B L H







__1/2" EXPANSION JOINT

4" COMPACTED

SUBGRADE

COMPACTED

BASE COURSE

_ EXIST. SIDEWALK



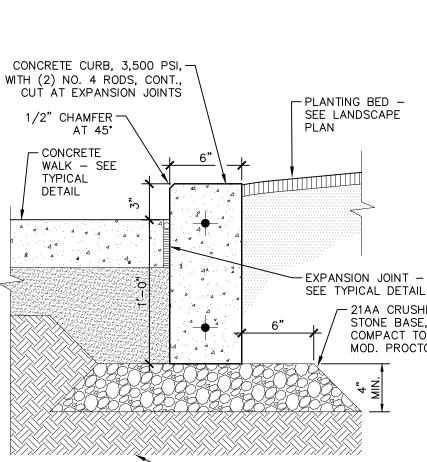
1. SET 1/2" EXPANSION JOINTS AT POINTS ABUTTING CURB OR PAVEMENT.

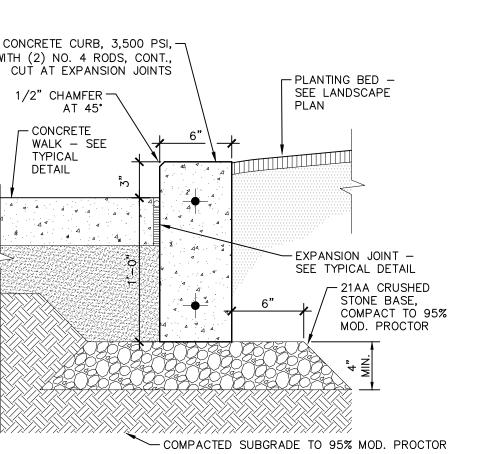
THICKNESS DEEP. CONTROL JOINTS SHALL BÉ SAWED OR TOOLED TO

2. CONTROL JOINTS SHALL BE 1/8" WIDE AND 1/4 OF THE SIDEWALK

-4" PCC (CONCRETE) SIDEWALK

CROSS SLOPE AT 2% MAX





TYPE OF JOINT USE DESCRIPTION DETAIL A. CONTROL JOINT. PLACE JOINT SO AS TO JOINT SEALANT 3/4" UNIFORMLY DIVIDE SLAB DEEP (MIN.) HELD 1/4" SAWED JOINT AS SHOWN ON DETAIL BELOW SURFACE SEALED WITH HOT-POURED RUBBER-ASPHALT 141/4" 4 4 4 4 4 4 JOINT SEALING COMPOUND - JOINT SEALANT 3/4" B. EXPANSION JOINT PLACE AGAINST EXISTING DEEP (MIN.) HELD 1/4" CONCRETE PAVEMENTS, BELOW SURFACE STRUCTURES (BUILDINGS, CANOPY COLUMNS, EXISTING CONCRETE TRENCH DRAINS, ETC.) NEW WORK PAVEMENTS OR STRUCTURES ─ 1/2" EXPANSION MATERIAL CONSTRUCTION JOINTS C. CONSTRUCTION JOINT MUST BE PLACED WHEN 3/4" MIN. CONTINUOUS POUR OPERATIONS ARE ** NEW WORK 44 SUSPENDED FOR MORE THAN 30 MINUTES. →1:3 SLOPE

BUTT JOINT NOT TO SCALE

CONCRETE JOINT DETAILS NOT TO SCALE

ALIGN DRIVEWAY RETURN TO FIT OPENING IN CURB AND GUTTER FLOW LINE └-1" EXPANSION 1" EXPANSION JOINT WEAKNESS JOINTS GUTTER PAN *1'-6" + CURB & * TO EDGE OF GUTTER GUTTER WIDTH I GUTTER OR FACE OF INTEGRAL CURB PAVEMENT T THICKNESS 1 -REINFORCEMENT AS IN ADJACENT CURB & GUTTER **SECTION A-A** TYPE 'M'

DRIVEWAY OPENING AND APPROACH

PLANTER CURB NOT TO SCALE

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RELATIONSHIPS - REPUTATION - RESULTS 800.525.6016 1051-19-9457 08/26/2020 | Checked by | Drawn by | Crew/Book CB CB KMW AR BROUWER COMPANY **LEWIS JEWELERS** 300 S. MAPLE ROAD ANN ARBOR, MICHIGAN 48103 WASHTENAW ANN ARBOR 5E 25 2S

HRISTOPHER SUTTON, PE --

Sheet Number

12

TYPICAL DETAILS

SITE PLAN

Know what's **below**.

THE LOCATIONS OF EXISTING

JNDERGROUND UTILITIES ARE SHOWN

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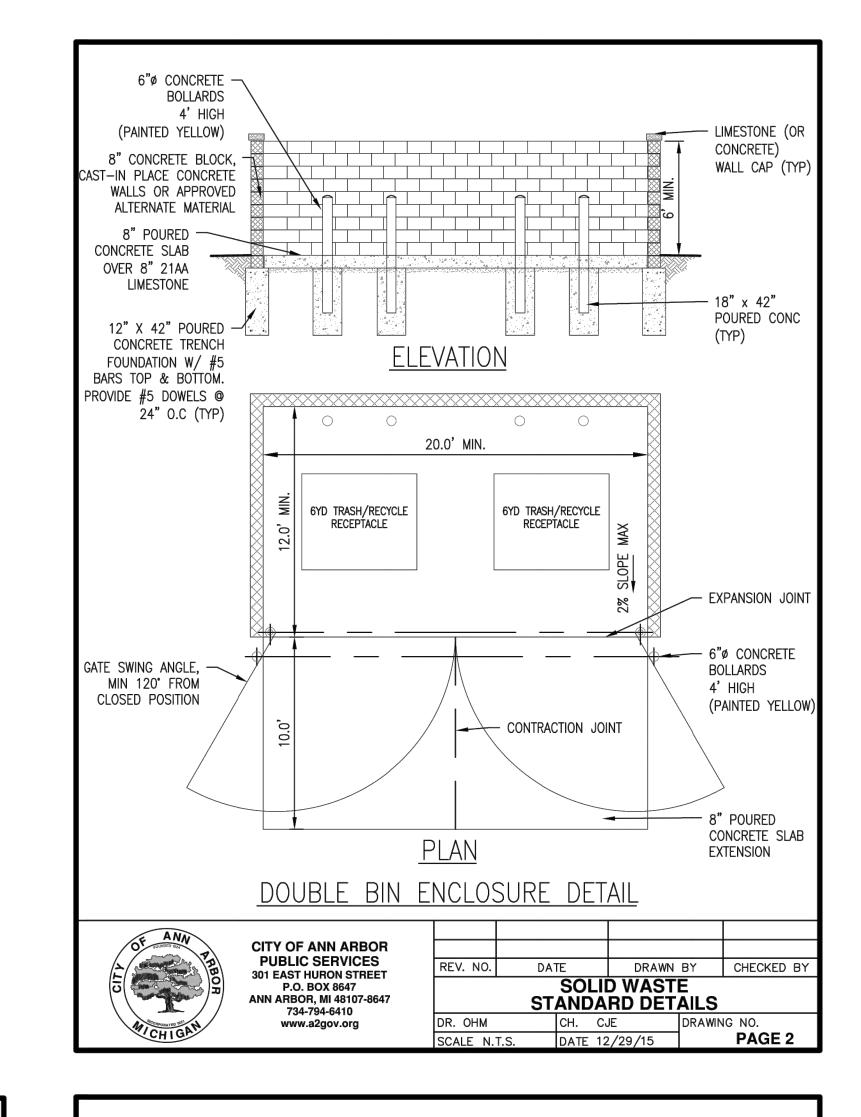
09.29.2020 \(\text{REVISED PER SITE PLAN REVIEW #1} \)

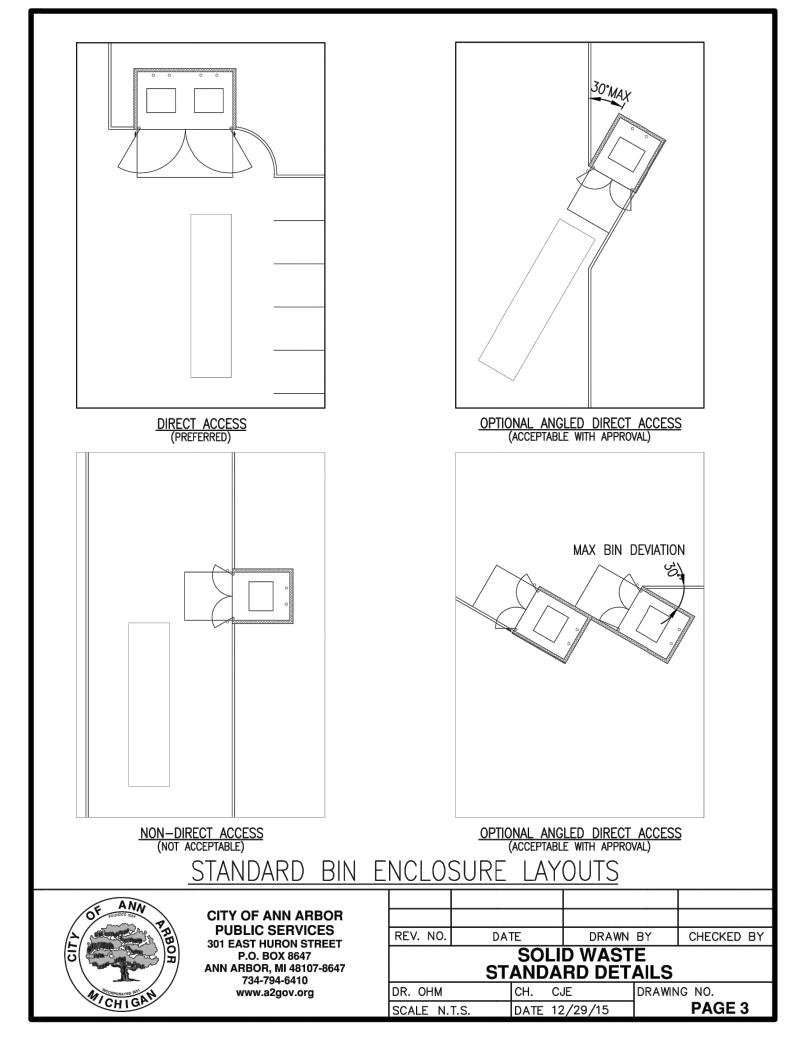
REVISED PER SITE PLAN REVIEW #2

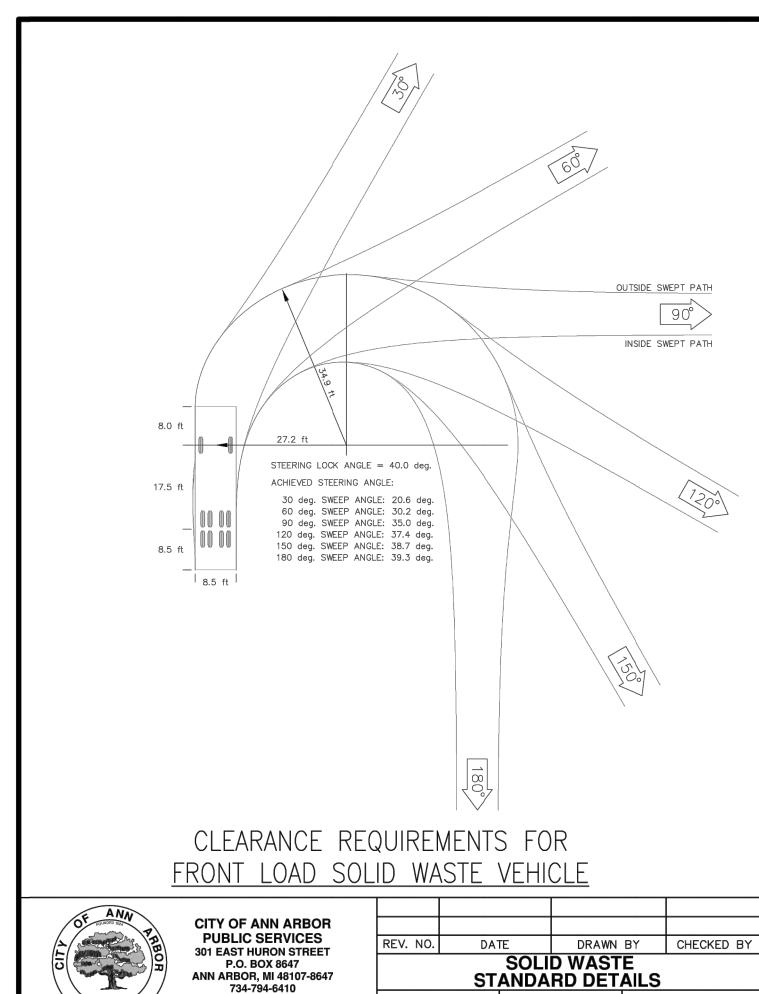
12.07.20
REVISED PER SITE PLAN REVIEW #3

08.27.2020 SITE PLAN SUBMISSION

Call before you dig.







DR. OHM

CH. CJE

DATE 12/08/15

DRAWING NO.

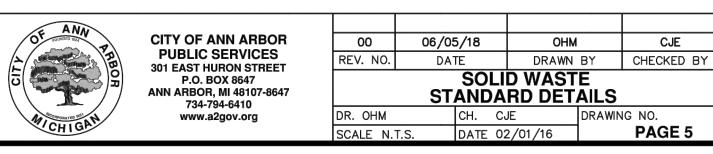
PAGE 4

GENERAL NOTES:

"NO PARKING", AS APPROVED BY CITY.

- 1. A CLEAR SPACE MUST BE MAINTAINED DIRECTLY IN FRONT OF THE SOLID WASTE ENCLOSURE. THE CLEAR SPACE SHALL BE A MINIMUM OF FIFTY (50) FEET LONG BY THE WIDTH OF THE INSIDE DIMENSION OF THE ENCLOSURE PLUS FOUR (4) FEET ON BOTH SIDES. A MINIMUM VERTICAL CLEARANCE OF AT LEAST TWENTY—FIVE (25) FEET MUST BE PROVIDED ABOVE THIS AREA.
- 2. INGRESS AND EGRESS ROUTES MUST BE DEVELOPED BASED ON SOLID WASTE SWEPT PATH REQUIREMENTS. A MINIMUM HORIZONTAL CLEARANCE OF TWO (2) FEET FROM THE EDGE OF THE SWEPT PATH AND A MINIMUM VERTICAL CLEARANCE OF AT LEAST FIFTEEN (15) FEET MUST BE PROVIDED ALONG THE ENTIRE ROUTE.
- 3. GATES ON ENCLOSURES MUST BE DESIGNED TO OPEN A MINIMUM OF 120 DEGREES FROM THE CLOSED POSITION. THE GATES MUST NOT REDUCE THE REQUIRED ENCLOSURE OPENING WIDTH, BLOCK ADJACENT PARKING SPOTS, OR BE IMPEDED BY ADJACENT CURBS OR LANDSCAPING.
- 4. GATES SHALL BE DESIGNED TO BE FREE STANDING WITHOUT A CENTER POLE. IF A CENTER POLE DESIGN IS NECESSARY, 12—INCHES SHALL BE ADDED TO THE OVERALL WIDTH OF THE ENCLOSURE.
- 5. GATE DESIGN SHALL INCLUDE A RELIABLE MEANS TO SECURE THE DOOR IN BOTH THE OPEN AND CLOSED POSITIONS.
- 6. THE CONCRETE SLAB IN FRONT OF THE BIN ENCLOSURE SHALL HAVE PAVEMENT MARKINGS TO INDICATE
- 7. THE PROPERTY OWNER SHALL BE RESPONSIBLE FOR THE INSTALLATION AND MAINTENANCE OF NO PARKING SIGNS ALONG THE SOLID WASTE INGRESS/EGRESS ROUTE TO ENSURE THE ROUTE REMAINS
- FREE OF OBSTRUCTIONS.

 8. REFER TO ASSOCIATED STANDARD DETAILS FOR REQUIREMENTS ON SINGLE AND DOUBLE WIDE SOLID
- WASTE BIN ENCLOSURE LAYOUT AND DESIGN CRITERIA. THE CITY SHALL HAVE THE ABILITY TO MODIFY OR INTERPRET THESE DETAILS AS NECESSARY, TO ACCOMMODATE THE CITY OR CITY CONTRACTOR NEEDS IN REGARDS TO SOLID WASTE PICK-UP.
- 9. SOLID WASTE ACCESS ROADS AND SERVICE AREA SURFACES SHALL BE DESIGNED AND MAINTAINED TO SUPPORT THE IMPOSED LOADS OF COLLECTION TRUCKS WEIGHING UP TO 66,000 LBS GROSS VEHICLE WEIGHT (GVW) AND SHALL BE PROVIDED WITH AN APPROVED SURFACE SO AS TO PROVIDE ALL WEATHER DRIVING CAPABILITIES. PROPERTY OWNER SHALL BE RESPONSIBLE FOR ALL SNOW AND ICE REMOVAL REQUIRED FOR SAFE ACCESS OF SOLID WASTE VEHICLES.
- 10. THE SOLID WASTE COLLECTION LOCATION SHALL BE LOCATED A MINIMUM OF TEN (10) FEET AWAY FROM MAJOR ELECTRICAL EQUIPMENT, ABOVE GROUND UTILITY SERVICES, TREE BRANCHES, BALCONIES OR OTHER OVERHEAD OBSTRUCTIONS.



GENERAL NOTES CONTINUED:

- 11. IF FORWARD ACCESS TO THE PUBLIC STREET IS NOT AVAILABLE FOR THE SOLID WASTE TRUCK, THE SITE DEVELOPMENT LAYOUT MUST ACCOMMODATE A TURN-AROUND LOCATION.
- 12. FOR SITES THAT CANNOT ACCOMMODATE A TURN-AROUND, THE FOLLOWING REQUIREMENTS MUST BE
- 12.1. SOLID WASTE TRUCKS MUST BE ABLE TO SERVICE DUMPSTERS WITHOUT IMPEDING THE PUBLIC STREET OR SIDEWALK
- 12.2. THE DUMPSTER COLLECTION LOCATION SHALL BE CLEARLY DELINEATED AND NOT HAVE A SLOPE GREATER THAN 2% IN ANY DIRECTION.
- 12.3. BOLLARDS OR ADEQUATE CLEAR SPACE MUST BE PROVIDED BEHIND THE LIFT POINT SO THE
- DUMPSTERS ARE NOT POTENTIALLY PUSHED INTO ANY BUILDING OR ACCESS ROUTE.

 12.4. ALL SWEPT-PATH CLEARANCE REQUIREMENTS PREVIOUSLY IDENTIFIED SHALL BE PROVIDED.
- 12.5. A VERTICAL CLEARANCE OF 25 FEET SHALL BE PROVIDED ABOVE THE COLLECTION LOCATION.
- 13. FOR SITES THAT CANNOT ACCOMMODATE A STANDARD DUMPSTER ENCLOSURE, THE DUMPSTERS MAY BE ROLLED OUT OF A BUILDING OR ALTERNATE ENCLOSURE BY THE PROPERTY OWNER TO A PROPOSED COLLECTION LOCATION. A COLLECTION SCHEDULE FOR RECYCLING AND TRASH COLLECTION SHALL BE PROVIDED ON THE SITE PLAN.
- 14. FOR SITES ADJACENT TO A PUBLIC ALLEY, SOLID WASTE TRUCKS ARE PERMITTED TO ACCESS THE PROPERTY THROUGH THE ALLEY IF SWEPT-PATH CLEARANCE REQUIREMENTS CAN BE PROVIDED.
- 15. SOLID WASTE COLLECTION LOCATIONS MUST BE LOCATED WITHIN THE BOUNDARIES OF THE PROPERTY.



CITY OF ANN ARBOR
PUBLIC SERVICES
301 EAST HURON STREET
P.O. BOX 8647
ANN ARBOR, MI 48107-8647
734-794-6410
www.a2gov.org

00 06/05/18 OHM CJE

REV. NO. DATE DRAWN BY CHECKED BY

SOLID WASTE

STANDARD DETAILS

DR. OHM CH. CJE DRAWING NO.

SCALE N.T.S. DATE 02/01/16 PAGE 6

Know what's below.
Call before you dig.

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RELATIONSHIPS - REPUTATION - RESULTS

800.525.6016

Project Number | Date | 1051-19-9457 | 08/26/2020 | P.M. | Checked by | Drawn by | Crew/Book

Client

AR BROUWER COMPANY

CB CB KMW

Dura tarak

LEWIS JEWELERS

Fite Address

300 S. MAPLE ROAD
ANN ARBOR, MICHIGAN 48103

County
WASHTENAW
Community
ANN ARBOR

Township Range Section
2S 5E 25

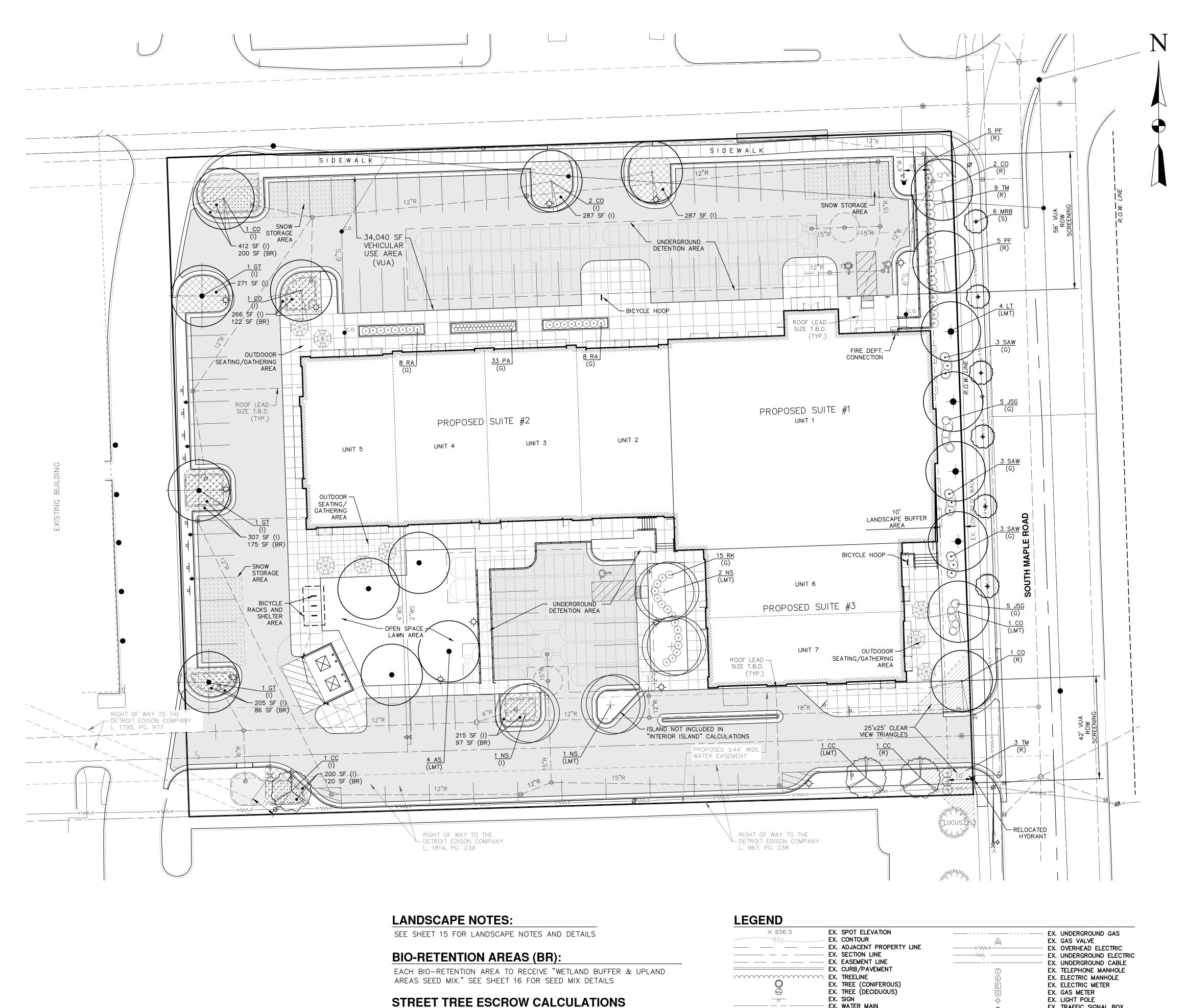
Professional Name | Date | Dat

SITE PLAN

TYPICAL DETAILS

Sheet Number

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THE CURRENT STREET TREE ESCROW RATE IS \$1.30 PER LINEAL FOOT OF

STREET FRONTAGE. BASED ON THE 270.36 LINEAL FEET OF SOUTH MAPLE

ROAD FRONTAGE ON THIS PROJECT, THE REQUIRED STREET TREE ESCROW

THIS AMOUNT MUST BE PAID PRIOR TO ISSUING THE BUILDING PERMIT. THE

DEVELOPER HAS PROVIDED THE REQUIRED STREET TREES FOR THIS SITE.

FORESTRY DIVISION HAS APPROVED THE STREET TREE PLANTINGS.

THEREFORE THE ESCROW AMOUNT SHALL BE REFUNDED AFTER THE CITY

FOR THIS SITE IS \$351.47

HACKBERRY					
RIGHT-OF-WAY SCREENING (R) REY	ΡΙ Δ	NT I	IST		
REY					
CC			, ,		
HACKBERRY	KEY	QTY.	SPECIES	MIN. SIZE	SPEC.
MALUS 'RED BARRON' 2" Cal. B&B MADMARK MITIGATION TREES (LMT) KEY QTY. SPECIES MIN. SIZE SBB MACKBERRY CC 1	СО	3		2" Cal.	B&B
TAM	СС	1		2" Cal.	B&B
DENSE YEW	-	4	-		
SHUBBY CINQUEFOIL	TM	12		30" Ht.	B&B
SPECIES	PF	10		30" Ht.	CONT
STREET TREES IN PUBLIC RIGHT-OF-WAY (S) REP	NTERI	OR LAN	IDSCAPE ISLANDS (I)		
THORNLESS HONEY LOCUST CO	KEY	QTY.	SPECIES	MIN. SIZE	SPEC.
HACKBERRY	GT	3		2" Cal.	B&B
CC	СО	4		2" Cal.	В&В
NS 1 NYSSA SYLVATICA BLACKGUM 9 PROVIDED STREET TREES IN PUBLIC RIGHT-OF-WAY (S) KEY QTY. SPECIES MIN. SIZE SPEC RED BARRON CRABAPPLE 6 PROVIDED ANDMARK MITIGATION TREES (LMT) KEY QTY. SPECIES MIN. SIZE SPEC AS 4 ACER SACCHARUM 2" Cal. B&B SUGAR MAPLE CC 1 CARPINUS CAROLINIANA 2" Cal. B&B AMERICAN HOP HORNBEAM CO 1 CELTIS OCCIDENTALIS 2" Cal. B&B HACKBERRY NS 3 NYSSA SYLVATICA 2" Cal. B&B BLACKGUM LT 4 LIRIODENDRON TULIPIFERA 2" Cal. B&B TULIP TREE 13 PROVIDED SENERAL PLANTINGS (G) KEY QTY. SPECIES MIN. SIZE SPEC SEA GREEN JUNIPER PA 33 PENNISETUM A. 'HAMELN' DWARF FOUNTAIN GRASS RA 31 RHUS AROMATICA 'GRO-LOW' 24" HT. & SPR. CONT	СС	1	CARPINUS CAROLINIANA	2" Cal.	B&B
TREET TREES IN PUBLIC RIGHT-OF-WAY (S) KEY QTY. SPECIES MIN. SIZE SPECE MRB 6 MALUS 'RED BARRON' 2" Cal. B&B RED BARRON CRABAPPLE 6 PROVIDED ANDMARK MITIGATION TREES (LMT) KEY QTY. SPECIES MIN. SIZE SPECE AS 4 ACER SACCHARUM 2" Cal. B&B SUGAR MAPLE CC 1 CARPINUS CAROLINIANA 2" Cal. B&B AMERICAN HOP HORNBEAM CO 1 CELTIS OCCIDENTALIS 2" Cal. B&B HACKBERRY NS 3 NYSSA SYLVATICA 2" Cal. B&B BLACKGUM LT 4 LIRIODENDRON TULIPIFERA 2" Cal. B&B TULIP TREE 13 PROVIDED SENERAL PLANTINGS (G) KEY QTY. SPECIES MIN. SIZE SPECE SENERAL PLANTINGS (G) KEY QTY. SPECIES MIN. SIZE SPECE 13 PROVIDED SENERAL PLANTINGS (G) KEY QTY. SPECIES MIN. SIZE SPECE SEA GREEN JUNIPER PA 33 PENNISETUM A. 'HAMELN' 1 GAL. CONT DWARF FOUNTAIN GRASS RA 31 RHUS AROMATICA 'GRO-LOW' 24" HT. & SPR. CONT	NS	1	NYSSA SYLVATICA	2" Cal.	B&B
KEY QTY. SPECIES MIN. SIZE SPECION MRB 6 MALUS 'RED BARRON' 2" Cal. B&B RED BARRON CRABAPPLE 6 PROVIDED AANDMARK MITIGATION TREES (LMT) KEY QTY. SPECIES MIN. SIZE SPECIES AS 4 ACER SACCHARUM SUGAR MAPLE 2" Cal. B&B CC 1 CARPINUS CAROLINIANA AMERICAN HOP HORNBEAM 2" Cal. B&B CO 1 CELTIS OCCIDENTALIS 2" Cal. B&B HACKBERRY NYSSA SYLVATICA BLACKGUM 2" Cal. B&B LT 4 LIRIODENDRON TULIPIFERA TULIPI FREE PROVIDED 2" Cal. B&B GENERAL PLANTINGS (G) KEY QTY. SPECIES MIN. SIZE SPECIES JSG 10 JUNIPERUS 'SEA GREEN' 24" HT. & SPR. CONT SEA GREEN JUNIPER 24" HT. & SPR. CONT DWARF FOUNTAIN GRASS RA 31 RHUS AROMATICA 'GRO-LOW' 24" HT. & SPR. CONT	-	9	_		
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DWARF FOUNTAIN GRASS RA 31 RHUS AROMATICA 'GRO-LOW' 24" HT. & SPR. CONT	JSG		JUNIPERUS 'SEA GREEN'	24" HT. & SPR.	CONT
RA 31 RHUS AROMATICA 'GRO-LOW' 24" HT. & SPR. CONT	PA	33		1 GAL.	CONT
	RA	31	RHUS AROMATICA 'GRO-LOW'	24" HT. & SPR.	CONT

MULCH

AREAS OF INDIVIDUAL TREES AND LANDSCAPE BEDS ARE TO BE MULCHED WITH A MINIMUM OF 4 INCHES OF SHREDDED HARDWOOD BARK.

24" HT. & SPR. CONT.

13 TREES 13 TREES

REQUIRED PROVIDED

\$351.47 **\$351.47**

NO GROUND WOOD PALETTE MULCH PERMITTED

50% CAL. INCH OF LANDMARK TREE REMOVED

270.36 LF SOUTH MAPLE ROAD STREET FRONTAGE

STREET TREE ESCROW

\$1.30 / LF OF STREET FRONTAGE

\$1.30 X 270.36 LF = \$351.47

PORTION OF BIO—RETENTION AREA NOT APPLYING TO REQUIREMENTS

SNOW STORAGE AREA

12" & 14" LANDMARK CRABAPPLE TO BE REMOVED 26" LANDMARK TREE REMOVED X 50% =

ANTHONY WATERER SPIREA

SAW 9 SPIRAEA X BUMALDA

LANDSCAPE REQUIREMENT CALCULATIONS

5.20.3.A. VEHICULAR USE - RIGHT-OF-WAY SCREENING (R)	REQUIRE	<u>:D</u>	PROVIDE	<u></u>
VUA R.O.W. SCREENING LENGTH = 56 + 42 = 98 LF				
1 TREE / 30 LF REQUIRED			1	
98 LF \ 30 LF = 3.3 = 4 TREES REQUIRED	4		4	
5.20.3.B VEHICULAR USE / INTERIOR LANDSCAPE ISLANDS (I)	<u>require</u>	<u>D</u>	PROVIDE	<u>D</u>
VEHICULAR USE AREA = 31,987 SF	[1	
INTERIOR LANDSCAPE AREA REQUIRED	[1	
3,300 - 49,999 SF = 1:20 SF			1	
31,987 SF \ 20 SF = 1,702 SF	[1	
INTERIOR LANDSCAPE AREA REQUIRED	1,599	SF	2,450	SF
1 TREE REQUIRED / 250 SF OF AREA			1	
1,599 SF \ 250 SF = 6.4 = 7 TREES REQUIRED	7	EA	9	EA
DEPRESSED BIORETENTION AREA REQUIRED	1		1	
1,599 SF > 750 SF			1	
1,599 SF X 50% =	800	SF	800	SF
5.20.4 CONFLICTING LAND USE BUFFERS	NOT AP	PLIC	ABLE	
5.20.5 PRIVATE STREETS AND SHARED DRIVEWAY BUFFERS	NOT AP	PLIC	ABLE	
5.20.10. STREET TREES IN THE PUBLIC RIGHT-OF-WAY (S)	REQUIRE	<u>D</u>	PROVIDE	<u> D</u>
R.O.W. WAY LENGTH = 270.36 LF	1		1	
1 TREE / 45 LF REQUIRED	1		1	
270.36 LF \ 45 LF = 6 TREES REQUIRED	6		6	
5.29.6.F.4 NATURAL FEATURE MITIGATION (LMR)	<u>REQUIRE</u>	<u>:D</u>	PROVIDE	<u>D</u>
LANDMARK TREE REPLACEMENT REQUIREMENT:				



THE LOCATIONS OF EXISTING UNDERGROUND UTILITIES ARE SHOWN IN AN APPROXIMATE WAY ONLY AND HAVE NOT BEEN INDEPENDENTLY VERIFIED. THE CONTRACTOR SHALL DETERMINE THE EXACT LOCATION OF ALL EXISTING UTILITIES BEFORE COMMENCING WORK, AND AGREES T BE FULLY RESPONSIBLE FOR ANY AND ALL DAMAGES WHICH MIGHT BE OCCASIONED BY THE CONTRACTOR'S FAILURE TO EXACTLY LOCATE AND PRESERVE ANY AND ALL UNDERGROUND UTILITIES.

CONSTRUCTION SITE SAFETY IS THE RESPONSIBILITY OF THE CONTRACTOR. NEITHER THE OWNER NOR THE ENGINEER SHALL BE EXPECTED TO ASSUME ANY RESPONSIBILITY FOR SAFETY OF THE WORK, OF PERSONS ENGAGED IN THI WORK, OF ANY NEARBY STRUCTURES, OR OF ANY OTHER PERSONS.

08.27.2020 SITE PLAN SUBMISSION 09.29.2020
REVISED PER SITE PLAN REVIEW #1

10.22.2020 REVISED PER SITE PLAN REVIEW #2 12.07.20
REVISED PER SITE PLAN REVIEW #3

RELATIONSHIPS - REPUTATION - RESULTS

800.525.6016

1051-19-9457 08/26/2020 P.M. | Checked by | Drawn by | Crew/Book | CB | KMW | .

AR BROUWER COMPANY

LEWIS JEWELERS

300 S. MAPLE ROAD ANN ARBOR, MICHIGAN 48103 ANN ARBOR WASHTENAW TownshipRangeSection2S5E25

CHRISTOPHER SUTTON, PE --

SITE PLAN LANDSCAPE PLAN

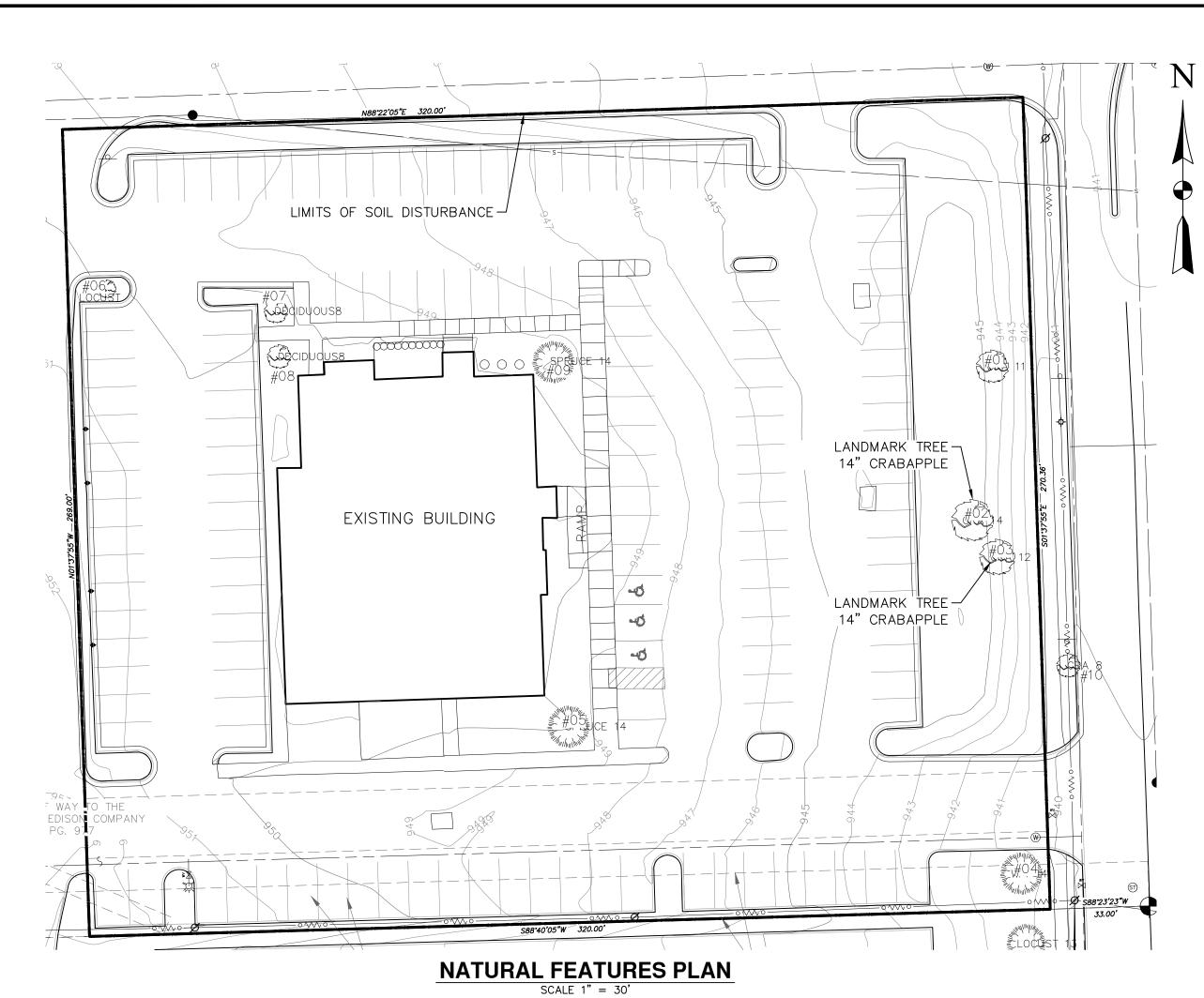
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× 656.5	EX. SPOT ELEVATION	EX. UNDERGROUND GAS
652———	EX. CONTOUR	$^{\mathbb{G}}$ EX. GAS VALVE
	EX. ADJACENT PROPERTY LINE	
	EX. SECTION LINE	
	EX. EASEMENT LINE	EX. UNDERGROUND CABLE
	EX. CURB/PAVEMENT	T EX. TELEPHONE MANHOLE
·	EX. TREELINE	E EX. ELECTRIC MANHOLE
0	EX. TREE (CONIFEROUS)	E EX. ELECTRIC METER
Ö	EX. TREE (DECIDUOUS)	© EX. GAS METER
	EX. SIGN	
	EX. WATER MAIN	-;- EX. LIGHT POLE ■ EX. TRAFFIC SIGNAL BOX
W	EX. WATER VALVE	
	EX. HYDRANT	© EX. UNIDENTIFIED MANHOLE
₩		Ø EX. TRAFFIC SIGNAL POLE
(M) (M) (M)	EX. WATER MANHOLE	
<u> </u>	EX. WELL	——————————————————————————————————————
$\overline{\mathbb{W}}$	EX. WATER METER	PROPOSED BUILDING
	EX. STORM SEWER	INTERIOR LANDSCAPE ISLAND AREA
CB	EX. STORM INLET/CATCH BASIN	
(ST)	EX. STORM MANHOLE	PORTION OF BIO-RETENTION AREA APPLYING TO REQUIREMENTS
\smile	EX. STORM END SECTION	AFFLIING TO REQUIREMENTS

EX. SANITARY SEWER

EX. STORM END SECTION

EX. SANITARY MANHOLE



NATURAL FEATURES PLAN NOTES

- . LIMITS OF DISTURBANCE ARE SHOWN ON THE PLAN
- 2. NO ENDANGERED SPECIES HABITAT EXIST ON THIS SITE.
- S. PER THE ALTA SURVEY: THE PARCEL DOES NOT LIE WITHIN A SPECIAL FLOOD HAZARD ZONE ACCORDING TO THE FLOOD INSURANCE RATE MAP PUBLISHED BY THE FEDERAL INSURANCE ADMINISTRATION FOR THE COUNTY OF WASHTENAW, STATE OF MICHIGAN, COMMUNITY NUMBER 26013, MAP NUMBER 26161C0244E, AN EFFECTIVE DATE OF APRIL 3, 2012. THEREFORE, AS NOTED IN SAID DOCUMENT, THE SCALED LOCATION OF THE EASTERLY PORTION OF THE SUBJECT PROPERTY IS IN ZONE X (UNSHADED),
- THE PARCEL DOES NOT LIE WITHIN A SPECIAL FLOOD HAZARD ZONE ACCORDING TO THE FLOOD INSURANCE RATE MAP INDEX PUBLISHED BY THE FEDERAL INSURANCE ADMINISTRATION FOR THE COUNTY OF WASHTENAW, STATE OF MICHIGAN, MAP NUMBER 26161CINDOA, AN EFFECTIVE DATE OF APRIL 3, 2012. THEREFORE, AS NOTED (*PANEL NOT PRINTED NO SPECIAL FLOOD HAZARD AREAS) IN SAID DOCUMENT, THE SCALED LOCATION OF THE WESTERLY PORTION OF THE SUBJECT PROPERTY RESIDES IN PANEL 26161C0243E.
- 4. TWO LANDMARK TREES EXIST ON THIS SITE AND WILL BE REMOVED. THEIR LOCATION AND CONDITION IS NOTED ON THE PLAN.
- 5. NO STEEP SLOPES EXIST ON THIS SITE.
- 6. NO EXISTING OR PROPOSED WATER COURSE ARE ON THIS SITE.
- 7. NO WETLANDS EXIST ON THIS SITE.

NATURAL FEATURES - STATEMENT OF IMPACT

THE PROPOSED PROJECT INCLUDES THREE COMMERCIAL BUILDINGS WITH A MAXIMUM POTENTIAL CAPACITY OF 6 BUSINESSES.

THE EXISTING NATURAL FEATURES ON THE SITE ARE LIMITED TO TWO LANDMARK TREES AS FOLLOWS:

TREE #02 14" CAL. CRABAPPLE TREE, GOOD CONDITION
TREE HEALTH / CONDITION FACTOR SCORE = 21
TREE #03 12" CAL. CRABAPPLE TREE, GOOD CONDITION
TREE HEALTH / CONDITION FACTOR SCORE = 21

SUMMARY:

TREE #02 14" CAL. CRABAPPLE TREE — <u>REMOVE</u> DUE TO EXISTING LOCATION ONSITE FALLING WITH PROPOSED BUILDING — BUILDING LOCATION AS

REQUIRED BY ORDINANCE FOR FRONT SETBACK

TREE #03 12" CAL. CRABAPPLE TREE — REMOVE DUE TO EXISTING LOCATION ONSITE FALLING WITH PROPOSED BUILDING — BUILDING LOCATION AS REQUIRED BY ORDINANCE FOR FRONT SETBACK

EXISTING TREE LIST:

NO.	SIZE	SPECIES	STATUS
01	11" CAL.	CRAB APPLE TREE	TO BE REMOVED
02	14" CAL. (LM)	CRAB APPLE TREE	TO BE REMOVED
03	12" CAL. (LM)	CRAB APPLE TREE	TO BE REMOVED
04	14" CAL.	PINE TREE	TO BE REMOVED
05	14" CAL.	SPRUCE	TO BE REMOVED
06	3" CAL.	LOCUST TREE	TO BE REMOVED
07	8" CAL.	CRAB APPLE TREE	TO BE REMOVED
80	8" CAL.	CRAB APPLE TREE	TO BE REMOVED
09	14" CAL.	SPRUCE	TO BE REMOVED
10	8" CAL.	CRAB APPLE TREE	TO BE REMOVED

(LM) DESIGNATES ANN ARBOR LANDMARK TREE

LANDSCAPE NOTES:

- 1. THIS PLAN IS FOR PLANTING LOCATIONS ONLY.
- 2. SIZES SPECIFIED ARE MINIMUM SIZES TO BE INSTALLED.
- 3. IN THE EVENT THE PLANT LIST DOES NOT MATCH THE PLAN, THE PLAN SHALL TAKE PRECEDENCE.
- 4. ALL EXISTING TREES TO REMAIN THAT ARE DAMAGED DURING CONSTRUCTION SHALL BE REPLACED BY THE END OF THE FOLLOWING PLANTING SEASON.
- 5. THE LANDSCAPE CONTRACTOR SHALL BE RESPONSIBLE FOR VERIFICATION OF ALL UNDERGROUND AND OVERHEAD UTILITIES. PLANT MATERIAL IS TO BE LOCATED SUCH THAT IT WILL NOT INTERFERE WITH ANY UNDERGROUND OR OVERHEAD UTILITIES. PLANTINGS WITHIN 15 FEET OF A FIRE HYDRANT SHALL NOT EXCEED 6 INCHES IN HEIGHT.
- 6. PLANT TREES AND SHRUBS AT THE SAME GRADE LEVEL AT WHICH THEY WERE GROWN IN THE NURSERY. IF HEAVY CLAY SOILS ARE EVIDENT, PLANT TREES AND SHRUBS IN LEVEL AREAS HIGHER, APPROXIMATELY 1/4 OF THE ROOT BALL ABOVE THE GRADE.
- 7. REMOVE ALL TWINE, WIRE, NURSERY TREE GUARDS, TAGS AND INORGANIC MATERIAL FROM ROOT BALLS. PEEL BACK THE TOP 1/3 OF BURLAP FROM EARTH BALLS AND REMOVE ANY BURLAP AROUND TREE TRUNKS.
- 8. ALL LANDSCAPE AREAS SHALL BE EXCAVATED OF ALL BUILDING / CONSTRUCTION MATERIAL AND POOR SOILS TO A DEPTH OF 18"-24" AND BACKFILLED WITH GOOD, MEDIUM TEXTURED PLANTING SOIL.
- 9. ALL DISTURBED UNPAVED AREAS ARE TO BE SPREAD WITH A MINIMUM 4 INCHES OF TOPSOIL AND SODDED.
- 10. AREAS OF INDIVIDUAL TREES AND LANDSCAPE BEDS ARE TO BE MULCHED WITH A MINIMUM OF 4 INCHES OF SHREDDED HARDWOOD BARK. (NO GROUND WOOD PALETTE MULCH PERMITTED) MULCH SHALL BE SPREAD IN A 5-FOOT DIAMETER CIRCLE AROUND THE BASE OF EACH TREE, LEAVING A 6-INCH RADIUS CIRCLE OF BARE SOIL AROUND THE TRUNK OF THE TREE. ALL AREAS OUTSIDE OF THE MULCH RINGS SHALL BE SODDED. MULCH TO BE DOUBLE SHREDDED HARDWOOD BARK MULCH
- 11. IF AN APPROVED SPECIES IS NO LONGER ACCEPTABLE DUE TO SUCH THINGS AS INFESTATION OR DISEASE, A SUITABLE SIMILAR SPECIES SHALL BE USED AS REPLACEMENT. ANY PLANT SUBSTITUTIONS SHALL HAVE CITY OF ANN ARBOR APPROVAL PRIOR TO INSTALLATION.
- 12. RECOMMENDED PLANTING DATES ARE MARCH 1 TO MAY 15 FOR ALL MATERIALS AND OCTOBER 15 TO DECEMBER 15 FOR DECIDUOUS MATERIALS. PLANTINGS OUTSIDE THESE DATES SHALL HAVE PRIOR CITY OF ANN ARBOR APPROVAL, AND MAY REQUIRE SPECIAL TREATMENT, SUCH AS EXTRA WATERING OR MULCHING, TO INCREASE SURVIVAL POTENTIAL.
- 13. TREE "BALLED AND BURLAPPED" ROOT BALL SIZE SHALL BE A MINIMUM OF AT LEAST TEN TIMES THE TREE CALIPER SIZE.
- 14. ANY SPECIES DEVIATIONS FROM THE APPROVED SITE PLAN MUST BE APPROVED IN WRITING BY THE CITY OF ANN ARBOR PRIOR TO INSTALLATION.

LANDSCAPE MAINTENANCE PLAN

LANDSCAPING REQUIRED BY THE CITY OF ANN ARBOR SHALL BE MAINTAINED IN A HEALTHY, NEAT AND ORDERLY APPEARANCE, FREE FROM REFUSE AND DEBRIS. SPECIFIC LANDSCAPE MAINTENANCE INCLUDES, BUT IS NOT LIMITED TO, CUTTING, PRUNING, AND MOWING OF REQUIRED LANDSCAPE AREAS AND REMOVAL OF TRIMMINGS.

APPLICATIONS OF FERTILIZERS, BEYOND THE INITIAL TOPSOIL AND SEEDING, SHALL BE A FERTILIZER WITH CONTAINING NO PHOSPHORUS.

ALL DISEASED, DAMAGED, OR DEAD PLANT MATERIAL SHALL BE REPLACED, IN ACCORDANCE WITH THE CITY OF ANN ARBOR'S UNIFIED DEVELOPMENT CODE, BY THE END OF THE FOLLOWING PLANTING SEASON AS A CONTINUING OBLIGATION FOR THE DURATION OF THE SITE PLAN.

THE OWNER SHALL INSURE PERPETUAL AND MANDATORY MAINTENANCE AND/OR REPLACEMENT OF VEGETATIVE PLANTINGS PURSUANT TO THE APPROVED LANDSCAPE PLAN.

IRRIGATION NOTE:

PROVIDE UNDERGROUND AUTOMATIC IRRIGATION SYSTEM. SYSTEM SHALL INCLUDE SEPARATE ZONES FOR LAWNS, SHRUBS PERENNIALS AND ANNUAL FLOWERS.

LANDSCAPE EDGING:

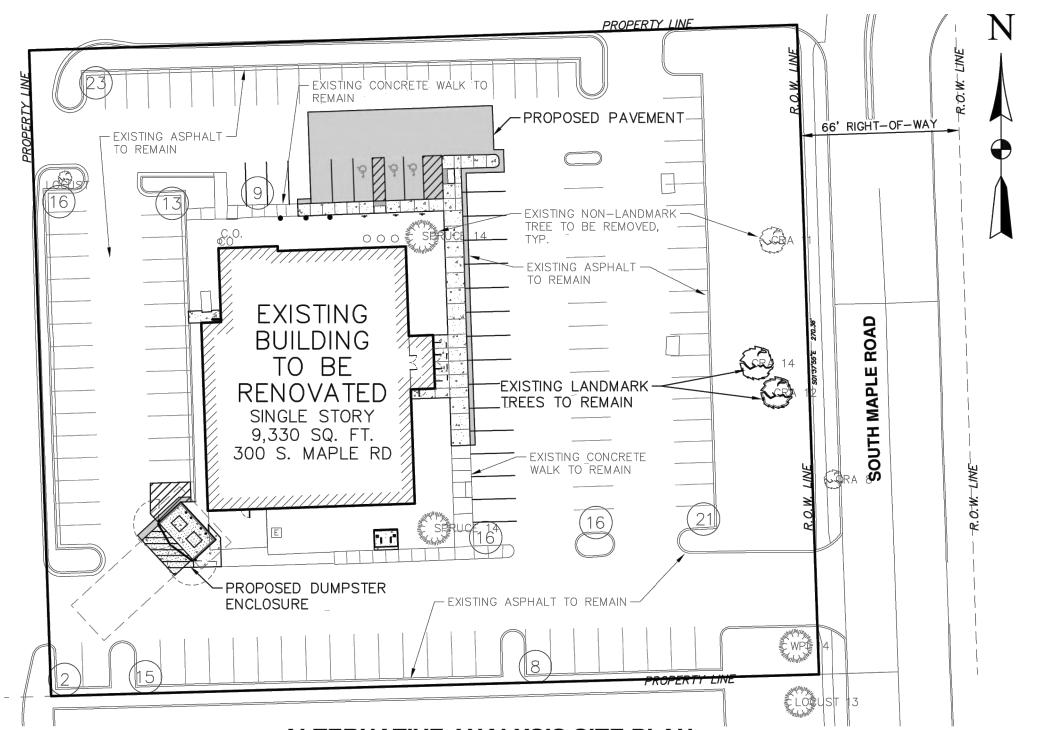
ALL LANDSCAPE EDGES ARE SHOVEL CUT.

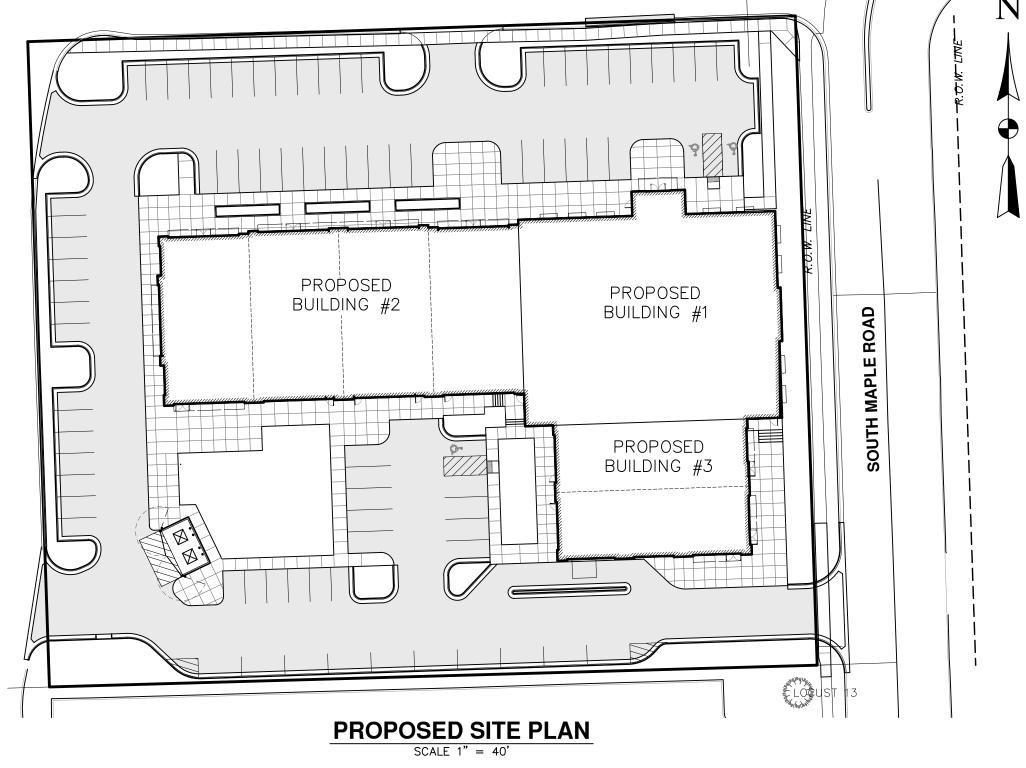
COMPACTED SOILS

- 1. NO MACHINERY IS TO BE USED OR PARKED WITHIN THE DRIPLINE OF EXISTING TREES.
- 2. PROPOSED PLANT BEDS FOUND TO HAVE COMPACTED SOILS SHALL BE FULLY EXCAVATED TO DEPTH SHOWN ON DETAILS. THESE AREAS (NOT INDIVIDUAL PLANT HOLES) ARE TO RECEIVE CONTINUOUS PLANT MIX AS SPECIFIED BELOW:
- A MIXTURE OF 70% LOAM TOPSOIL, 10% COMPOST, 20% SAND BACKFILLED IN THE PLANT BED.
- ADDITIONALLY HAND TILL TO A MINIMUM DEPTH OF 12", INTO EVERY 150 SQ. FT. BED AREA:
- (1) 6 CU. FT. BALE ORGANIC COMPOST
- (1) 40 LB BAG COMPOSTED POULTRY MANURE "CHICK MAGIC" 5-3-2 WWW.CHICKMAGIC.NET (262)495-6220
- (1) 5 LB BAG 13-13-13 MULTI-PURPOSE FERTILIZER

SNOW STORAGE NOTE:

SNOW SHALL NOT BE PUSHED ON TO INTERIOR LANDSCAPE ISLANDS UNLESS THOSE AREAS HAVE BEEN DESIGNATED FOR SNOW STORAGE.





ALTERNATIVE ANALYSIS SITE PLAN SCALE 1" = 40'

GROSS / NET LOT AREA		0.435 AC			
SITE DEVELOPMENT DATA		REQUIRED	EXISTING	PROPOSED	
ZON	ING	C3	C3	C3	
	FRONT	10'	159.64' EAST	155.93 EAST	
BUILDING	CIDE	0'	78.58' NORTH	84.89' NORTH	
SETBACKS	SIDE	U	75.95' SOUTH	75.95' SOUTH	
	REAR	20'	67.66' WEST	67.66' WEST	
BUILDING	HEIGHT	55 FT./4 Stories	+/-28'/1-STORY	28'/1-STORY	
INFORMATION	FLOOR AREA	6,000 SF	+/-9330 SF	9,330 SF	
PARKING	TOTAL SPACES	30 MIN.	142	139	
INFORMATION	INFORMATION ADA SPACES		3	3	

ALTERNATIVE ANALYSIS NARRATIVE

THE ALTERNATE PLAN IS THE ORIGINALLY PROPOSED RENOVATION OF THIS PROJECT. DUE TO THE CONDITION OF THE EXISTING BUILDING, TOTAL DEMOLITION OF THAT BUILDING WAS NECESSARY.

IN COMPARISON OF THE ALTERNATIVE AND THE PROPOSED PROJECTS, THE ALTERNATIVE PROPOSAL LIMITED THE USE AND AESTHETICS OF THE PROPERTY.

THE CITY'S CURRENT UNIFIED DEVELOPMENT CODE REQUIRES A 10-FOOT FRONT SETBACK FOR ALL PROPOSED BUILDINGS. CONSTRUCTING A BUILDING IN THE LOCATION OF THE PREVIOUS RESTAURANT WOULD REQUIRE A SETBACK VARIANCE FROM THE ZONING BOARD OF APPEALS.

ALTERNATIVE SITE PLAN PROS & CONS

FOR PROPOSED USE

- PRO ALLOWED FOR THE MAINTENANCE OF THE LANDMARK TREES
 CON PLAN MAINTAINED PAVEMENT, WALKS, AND LANDSCAPE
- WHICH EXIST IN POOR CONDITION.

 CON THE EXTERIOR OF THE BUILDING REQUIRED NUMEROUS

IMPROVEMENTS FOR THE HEALTH AND SAFETY OF THE EMPLOYEES

- AND CUSTOMERS

 ON DID NOT FULLY UTILIZE THE COMMERCIAL/TAXABLE
- ASPECTS OF THE PROPERTY

 ON PROVIDED SUBSTANTIALLY MORE PARKING THAN NECESSARY
- PROPOSED SITE PLAN PROS & CONS
- PRO PROVIDES SAFE AND HEALTHY ENVIRONMENT FOR EMPLOYEES AND CUSTOMERS
 PRO OVERALL ENHANCED AESTHETICS AND FUNCTIONALITY FOR THE COMMUNITY
 PRO FULLY UTILIZES THE POTENTIAL COMMERCIAL/TAXABLE REVENUE OF THE PROPERTY
 PRO PROVIDES ADEQUATE RATHER THAN EXCESSIVE PARKING FOR PROPOSED USE
- ACCESSING EXISTING COMMERCIAL USES WEST OF THIS SITE

 PRO PROVIDES FOR BOTH SHELTERED AND UN—SHELTERED BICYCLE PARKING

 PRO PROVIDES NEW AREAS FOR OUTDOOR SEATING AND GATHERING

PRO - INCLUDES NEW SIDEWALK FOR PEDESTRIAN CONNECTIVITY AND SAFETY IN

- PRO PROVIDES AREA FOR NATURAL BIO-RETENTION AREA
 PRO MEETS CITY DEVELOPMENT CODE FOR 10-FOOT BUILDING FRONT SETBACK.
 CON LOSE OF TWO LANDMARK TREES
- CON LOSE OF TWO LANDMARK TREES

GROSS / NET	LOT AREA	0.435 AC		
SITE DEVELOP	MENT DATA	REQUIRED	PROPOSED	
	FRONT	10'	10 EAST	
BUILDING	CIDE	O	73' NORTH	
SETBACKS	SIDE	0'	47' SOUTH	
	REAR	20'	53' WEST	
BUILDING #1	HEIGHT	55 FT/4 STORIES	28'/1-STORY	
INFORMATION	FLOOR AREA	6,000 SF	9,408 SF	
BUILDING #2	HEIGHT	55 FT/4 STORIES		
INFORMATION	FLOOR AREA	6,000 SF	10,575 SF	
BUILDING #3	HEIGHT	55 FT/4 STORIES		
INFORMATION	FLOOR AREA	6,000 SF	4,546 SF	
PARKING	TOTAL SPACES	30 MIN.	81	
INFORMATION	ADA SPACES	3 (MIN)	3	
BICYCLE PARKING	WITH SHELTER	4.5	6	
DICTULL PARKING	W/O SHELTER	4.5	4	



THE LOCATIONS OF EXISTING
UNDERGROUND UTILITIES ARE SHOWN
IN AN APPROXIMATE WAY ONLY AND
HAVE NOT BEEN INDEPENDENTLY
VERIFIED. THE CONTRACTOR SHALL
DETERMINE THE EXACT LOCATION OF
ALL EXISTING UTILITIES BEFORE
COMMENCING WORK, AND AGREES TO
BE FULLY RESPONSIBLE FOR ANY
AND ALL DAMAGES WHICH MIGHT BE
OCCASIONED BY THE CONTRACTOR'S
FAILURE TO EXACTLY LOCATE AND
PRESERVE ANY AND ALL

CONSTRUCTION SITE SAFETY IS THE RESPONSIBILITY OF THE CONTRACTOR. NEITHER THE OWNER NOR THE ENGINEER SHALL BE EXPECTED TO ASSUME ANY RESPONSIBILITY FOR SAFETY OF THE WORK, OF PERSONS ENGAGED IN THE WORK, OF ANY NEARBY STRUCTURES, OR OF ANY OTHER PERSONS.

UNDERGROUND UTILITIES.

08.27.2020 SITE PLAN SUBMISSION

09.29.2020
REVISED PER SITE PLAN REVIEW #1

10.22.2020
REVISED PER SITE PLAN REVIEW #2

12.07.20 REVISED PER SITE PLAN REVIEW #3

RELATIONSHIPS - REPUTATION - RESULTS
800.525.6016

AR BROUWER COMPANY

Project

LEWIS JEWELERS

Site Address

300 S. MAPLE ROAD

2S

ANN ARBOR, MICHIGAN 48103

County Community
WASHTENAW ANN ARBOR
Township Range Section

5E 25

Professional Name

HRISTOPHER SUTTON, PE --

SITE PLAN

LANDSCAPE NOTES, CALCULATIONS & NATURAL FEATURES

Sheet Number

BICYCLE PARKING

WETLAND BUFFER & UPLAND AREAS SEED MIX

Quote 19323

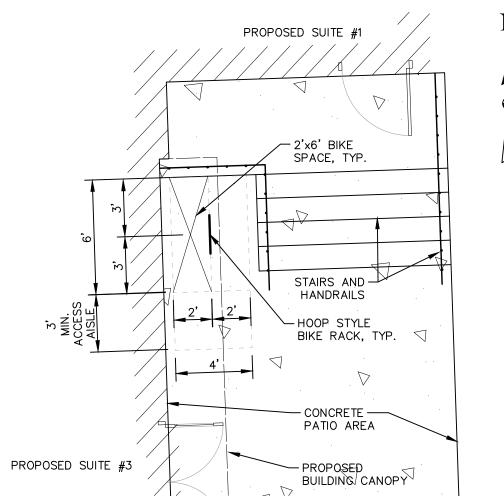
THE SEED MIX BELOW IS A CUSTOMIZED VERSION OF THE GENESIS NURSERY'S STANDARD "LO PRO PRAIRIE FOR CLAY SOLS" MIX

Metro Consulting no job name Salt Tolerant Sedge Ghetto Mix

Genesis Nursery, Inc. Tampico, Il

Illinois Permit # 3669 (877) 817–5325 www.GenesisNurseryInc.com

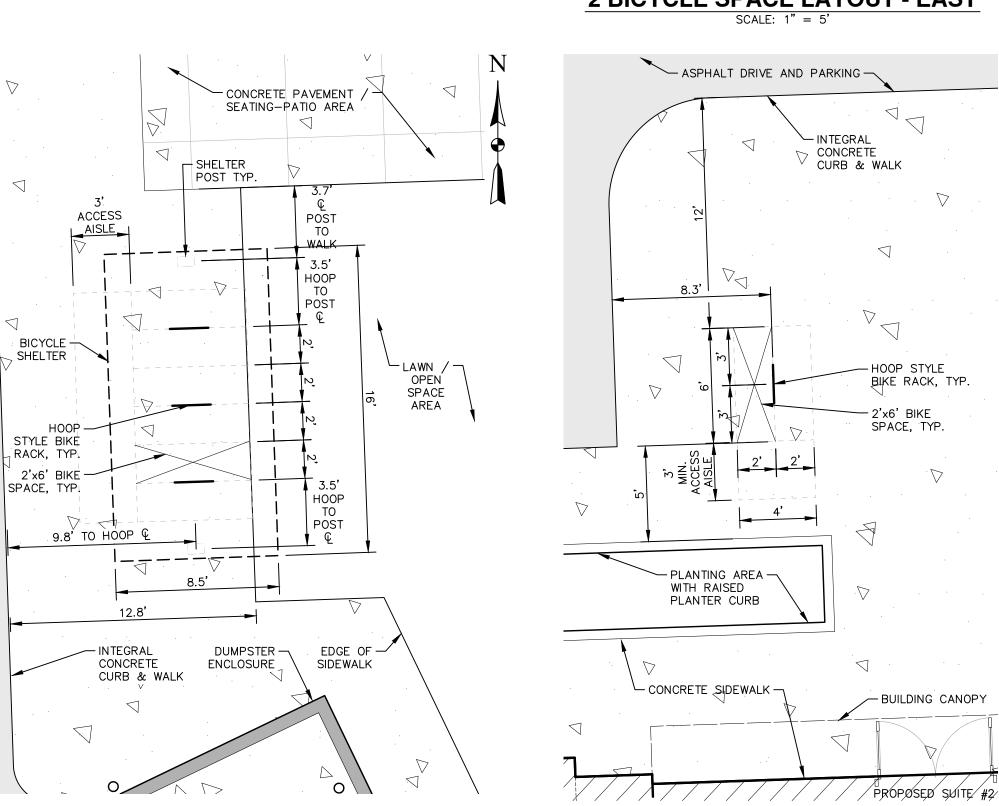
PLS where applicable		acre				
				permanent		
16-Jan-2020			seeds	matrix seeds	seeds	%
Species	Common Name	lb/ac	per pound	per acre	per sq. ft.	by sd ct
Agrostis stolonifera {A alba palustris}	Bent Grass	0.063	7,442,623	468,885	10.76	6.41%
Alisma subcordatum	Common Water Plantain	0.125	1,212,834	151,604	3.48	2.07%
Ammannia robusta	Scarlet Loosestrife	0.016	27,515,151	440,242	10.11	6.02%
Amorpha fruticosa	Indigo Bush	0.063	62,968	3,967	0.09	0.05%
Asclepias incarnata	Swamp Milkweed	0.125	63,148	7,894	0.18	0.11%
Bidens cernua	Nodding Burmarigold	0.063	305,660	19,257	0.44	0.26%
Bidens frondosa	Common Beggarstick	0.031	194,511	6,030	0.14	0.08%
Boltonia asteroides	False Aster	0.063	4,450,980	280,412	6.44	3.83%
Bulboschoenus fluviatilis (Scirpus f)	River Bulrush	0.125	123,571	15,446	0.35	0.21%
Bulboschoenus maritimus (Scirpus paludosus)	Alkali Bulrush	0.063	222,000	13,986	0.32	0.19%
Carex Bebbii	Bebbs Sedge	0.063	2,203,883	138,845	3.19	1.90%
Carex Iupuliformis	Knobbed Hop Sedge	0.125	97,758	12,220	0.28	0.17%
Carex nebraskensis	Nebraska Sedge	0.063	425,891	26,831	0.62	0.37%
Carex praegracilis	Expressway Sedge	0.125	637,640	79,705	1.83	1.09%
Carex scoparia	Pointed Broom Sedge	0.063	1,254,144	79,011	1.81	1.08%
Carex stipata	Awlfruited Sedge	0.063	650,430	40,977	0.94	0.56%
Carex vulpinoidea	Fox Sedge	0.500	1,964,030	982,015	22.54	13.43%
Cephalanthus occidentalis	Button Bush	0.125	112,043	14,005	0.32	0.19%
Distichlis spicata	Salt Grass	0.500	560,000	280,000	6.43	3.83%
Echinochloa crusgalli	Barnyard Grass	2.000	135,482	270,964	6.22	3.70%
Eleocharis obtusa	Blunt Spikerush	0.031	2,508,287	77,757	1.79	1.06%
Eleocharis palustris	Marsh Spikerush	0.063	1,181,250	74,419	1.71	1.02%
Elymus virginicus	Virginia Rye	5.000	117,252	586,260	13.46	8.02%
Eutrochium maculatum {Eupatorium m}	Spotted Joepyeweed	0.031	1,586,013	49,166	1.13	0.67%
Glyceria striata	Fowl Manna Grass	0.031	2,043,243	63,341	1.45	0.87%
Helenium autumnale	Sneezeweed	0.063	3,310,948	208,590	4.79	2.85%
Juncus arcticus balticus {J b littoralis}	Lake Shore Rush	0.016	9,659,574	154,553	3.55	2.11%
Juncus effusus	Soft Rush	0.016	22,700,000	363,200	8.34	4.97%
Juncus nodosus	Joint Rush	0.016	26,705,882	427,294	9.81	5.84%
Juncus torreyi	Torrey's Rush	0.016	23,402,062	374,433	8.60	5.12%
Leersia oryzoides	Rice Cut Grass	0.125	354,688	44,336	1.02	0.61%
Mimulus ringens	Monkey Flower	0.031	2,579,545	79,966	1.84	1.09%
Penthorum sedoides	Ditch Stonecrop	0.008	43,283,095	346,265	7.95	4.73%
Persicaria pensylvanica {Polygonum p}	Giant Smartweed	2.000	69,810	139,620	3.21	1.91%
Rudbeckia laciniata	Green Coneflower	0.063	219,536	13,831	0.32	0.19%
Sagittaria latifolia	Duck Potato	0.031	2,025,000	62,775	1.44	0.86%
Schoenoplectus acutus (Scirpus a)	Hardstem Bulrush	0.063	879,545	55,411	1.27	0.76%
Schoenoplectus pungens {Scirpus p}	Chairmakers Rush	0.125	189,246	23,656	0.54	0.32%
Schoenoplectus tabernaemontani {Scirpus va	li Great Bulrush	0.125	530,374	66,297	1.52	0.91%
Scirous atrovirens	Dark Green Rush	0.031	7.827.586	242,655	5.57	3.32%

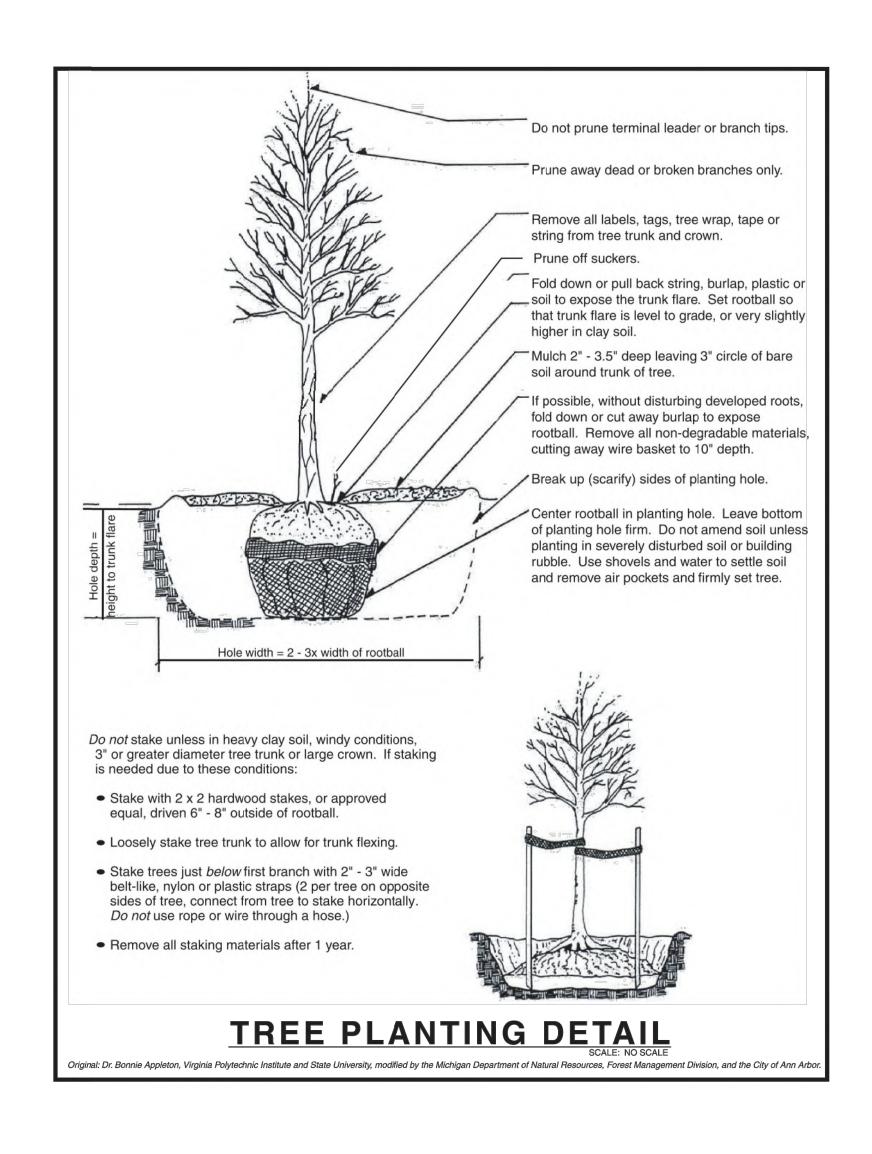


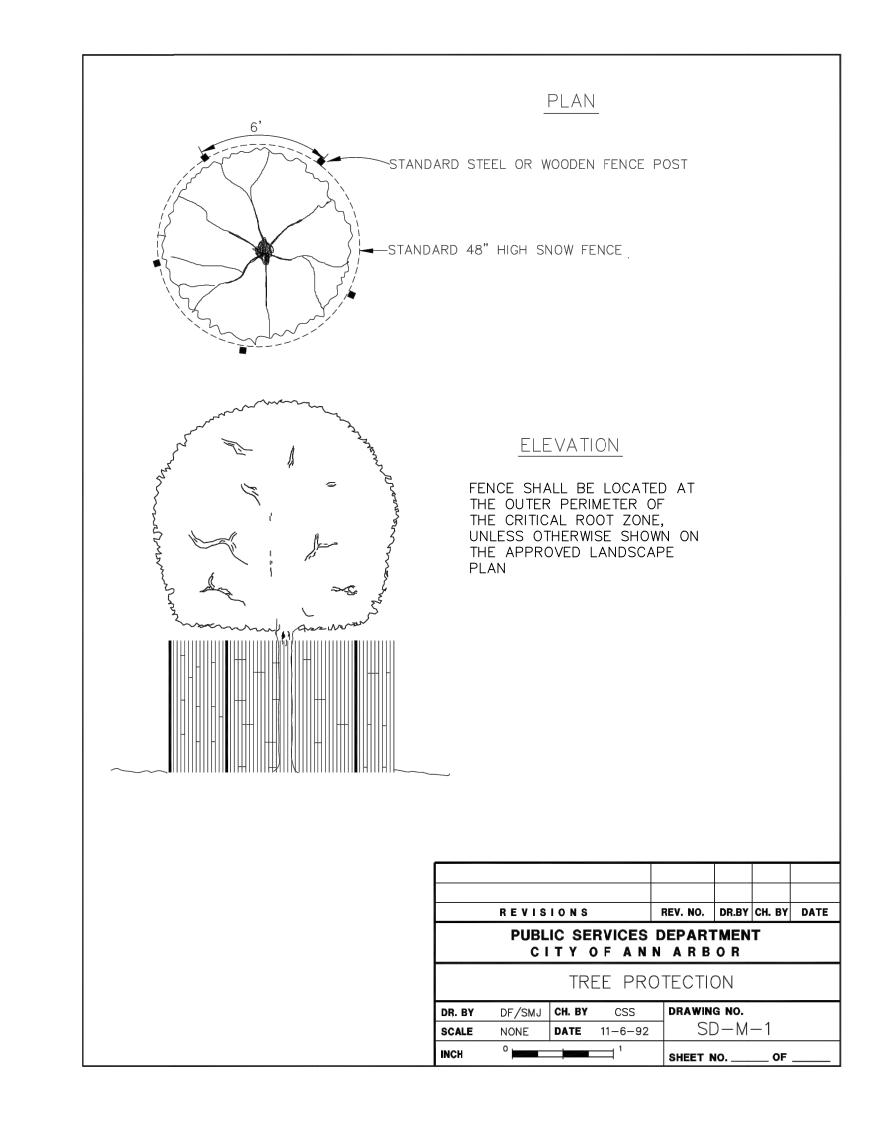
2 BICYCLE SPACE LAYOUT - EAST

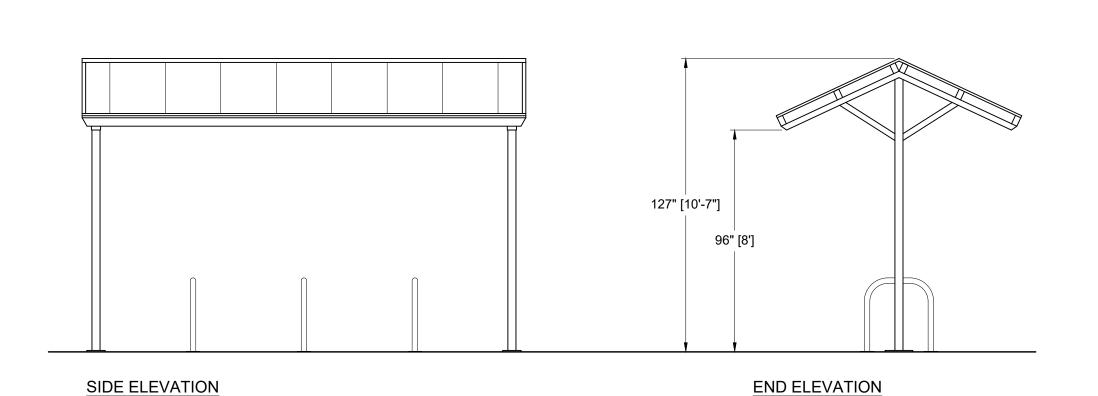
2 BICYCLE SPACE LAYOUT - NORTH

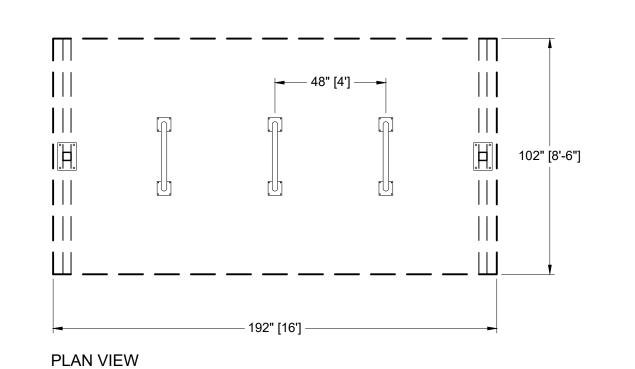
SCALE: 1" = 5'











48" RACK SPACING)

 FRAMEWORK TO BE WELDED AND MECHANICALLY FASTENED STEEL

 ALL FASTENERS TO BE STAINLESS STEEL ROOF GLAZING TO BE: 8MM POLYCARBONATE STRUCTURED SHEET, TONGUE AND GROOVE SYSTEM, IN ALUMINUM TRIM. TINT: _____

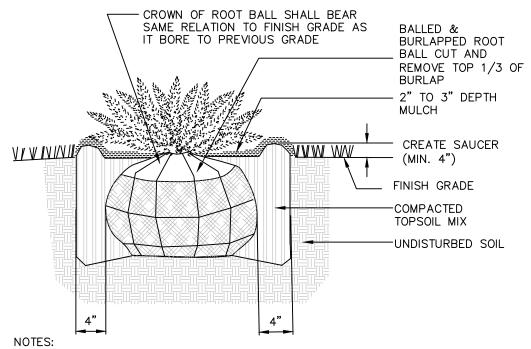
 STEEL FINISHING: MEDIA BLAST PREP

TNEMEC TNEME ZINC 90-97 PRIMER TNEMEC SERIES N69 HI-BUILD EPOXOLINE 2-PART EPOXY MIDCOAT;

TNEMEC SERIES 73 ENDURA-SHIELD TOPCOAT. ALUMINUM TRIM TO HAVE SAME TOP TWO COATS (OR IN MATCHING ANODIZED). COLOR: _____ ALL DIMENSIONS TO BE FIELD VERIFIED

 DESIGN IS PRELIMINARY, AND CONCEPTUAL, AND SUBJECT TO CHANGE BASED ON FINAL ENGINEERING PHASE AND CUSTOMER APPROVAL.

BIKE RACK SHELTER TO BE AS SHOWN, OR AN 8.5"x16' APEX' BICYCLE SHELTER BE AS SHOWN, OR APPROVED EQUAL



1. DO NOT ALLOW AIR POCKETS TO FORM WHEN BACKFILLING 2. WATER SHRUB THOROUGHLY SUBSEQUENT TO INSTALLATION

SHRUB PLANTING DETAIL NOT TO SCALE



BIKE RACK DETAIL



THE LOCATIONS OF EXISTING JNDERGROUND UTILITIES ARE SHOWN IN AN APPROXIMATE WAY ONLY AND HAVE NOT BEEN INDEPENDENTLY VERIFIED. THE CONTRACTOR SHALL DETERMINE THE EXACT LOCATION OF ALL EXISTING UTILITIES BEFORE COMMENCING WORK, AND AGREES TO BE FULLY RESPONSIBLE FOR ANY AND ALL DAMAGES WHICH MIGHT BE OCCASIONED BY THE CONTRACTOR'S FAILURE TO EXACTLY LOCATE AND PRESERVE ANY AND ALL UNDERGROUND UTILITIES.

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08.27.2020 SITE PLAN SUBMISSION

09.29.2020 \(\text{REVISED PER SITE PLAN REVIEW #1} \)

10.22.2020
REVISED PER SITE PLAN REVIEW #2 12.07.20
REVISED PER SITE PLAN REVIEW #3

RELATIONSHIPS - REPUTATION - RESULTS 800.525.6016

1051-19-9457 08/26/2020 C.M. | Checked by | Drawn by | Crew/Book | CB | KMW | .

AR BROUWER COMPANY

LEWIS JEWELERS

300 S. MAPLE ROAD ANN ARBOR, MICHIGAN 48103

WASHTENAW ANN ARBOR 2S 5E 25

CHRISTOPHER SUTTON, PE --

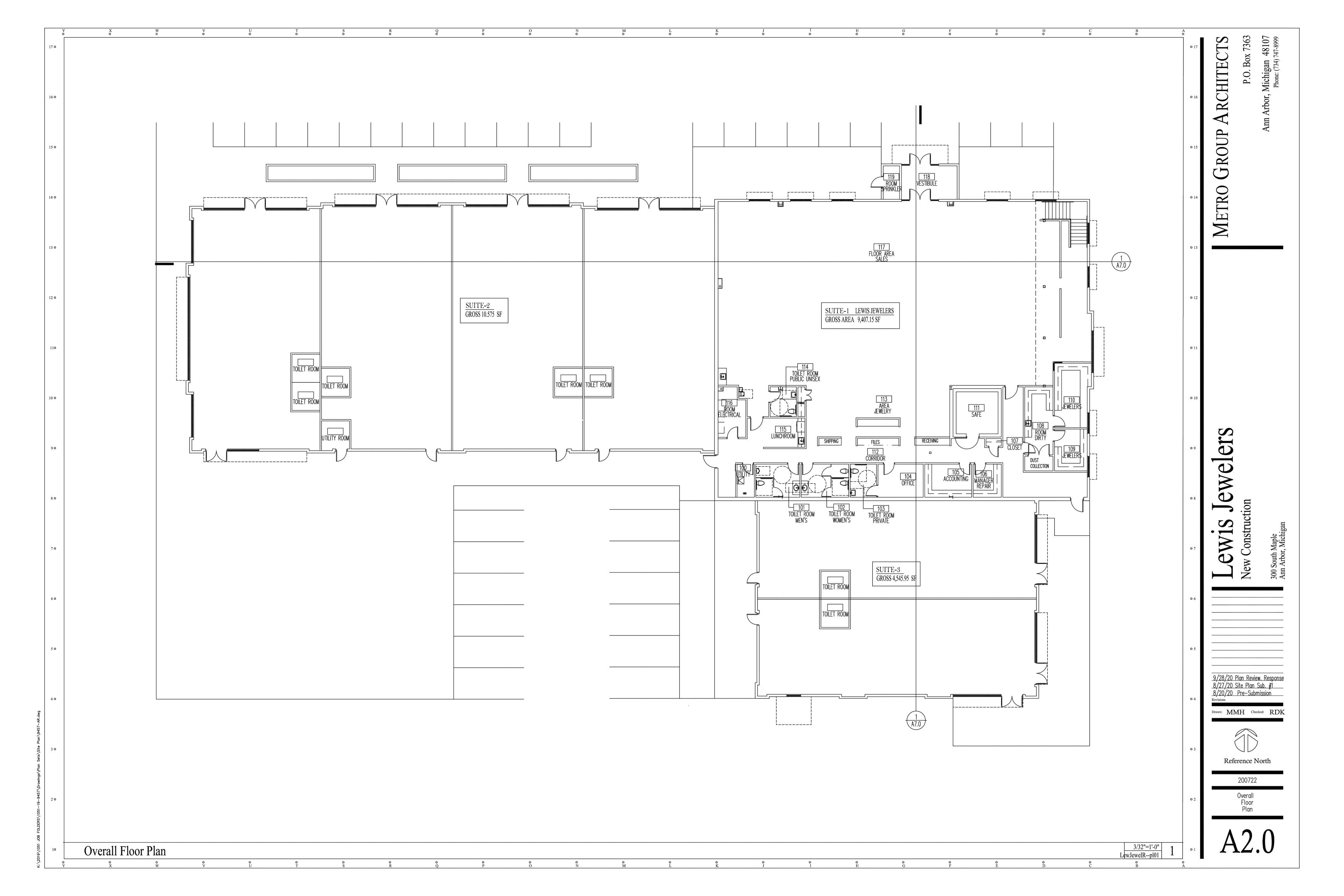
SITE PLAN LANDSCAPE DETAILS

Sheet Number

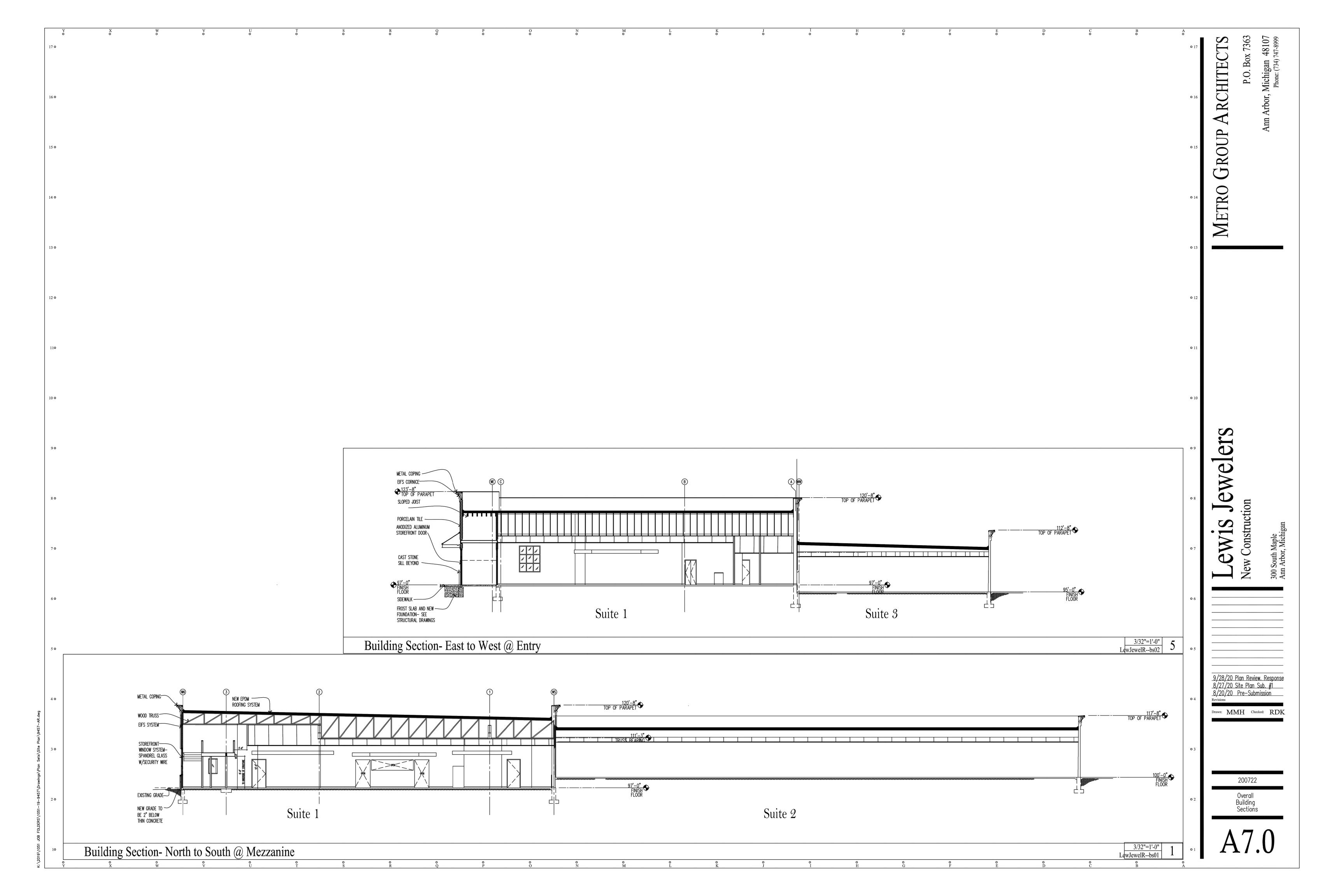
16

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6 BICYCLE SPACE WITH SHELTER LAYOUT









NORTH ELEVATION



WEST ELEVATION



SOUTH ELEVATION



EAST ELEVATION



SITE PLAN



OVERALL VIEW TO SOUTH WEST



OVERALL VIEW TO NORTH EAST



AREA PLAN

METRO GROU

ARCHITECTS

Lewis Jewelers

New Construction

300 S Ann A

9/28/2020 Site Plan Sub #2 8/27/20 Site Plan Sub. #1

wn: MMH Checked: RD

200722 Site and Building

Renderings

A8.0

6" W INV.: 939.92

PROPOSED SUITE #2

0.1 0.6 0.9 0.8 0.8 1.0 1.3 1.5 1.7 1.9 2.0 1.9 1.7 1.6 1.6 1.8 1.9 2.0 2.2 2.3 2.3 2.4 2.6 2.7 3.3 5.1 5.4 3.3 1.6 0.8 0.4 0.1

Plan View Scale - 1" = 20ft

1.6 2.5 2.3 1.4 2.5 2.3 1.2 0.

1.1 2.6 2.3 1.3 0.3 0.2 0.8 2.7 2.3 14 0.4 4.1 2.8

1.4 0.9 0.6 0.5 0.7 1.1 1.6 2.7 3.5 **C.@.20'**3.3 2.3 1.9 1.9 2.0 2.8 3.5 **3.2**

LO.5 2.9 2.9 1.5 1.0 0.7 0.5 0.7 1.1 1.6 2.2 2.5 2.4 2.0 1.7 1.8 1.8 2.3 2.6 2.5 2.5

0.4 3 2 3 3 1.6 1.0 0.8 0.6 0.8 1.3 1.9 2.4 2.2 2.0 1.9 1.7 1.7 1.6 1.9 2.1 2.3 1.9 0.6 0.7 0.7 0.4 0.2 0.4 2.0 2.6 1.6 1.2 1.3 1.7 2.3 3.5 5.7 6.3 4.6 3.2 2.8 2.5 2.5 2.8 3.8 5.8 6.8 2 4.9 3.3 2.2 2.0 1.8 1.8 1.8 1.7 1.5 1.2 0.8 0.4

0.1 1.1 1.6 1.3 1.1 1.3 1.8 2.2 2.7 3.5 3.6 3.2 2.6 2.5 2.4 2.6 2.7 3.1 4.0 4.0 3.6 3.2 3.2 2.7 2.8 3.1 3.3 2.6 1.9 1.5 0.9 0.4

0.0 0.3 0.4 0.4 0.4 0.5 0.5 0.6 0.6 0.6 0.6 0.6 0.6 0.6 0.7 0.8 0.8 0.8 0.8 0.8 0.8 0.9 1.0 1.2 1.3 1.3 1.1 0.8 0.3 0.1 0.0 0.0

Statistics									
Description	Symbol	Avg	Max	Min	Max/Min	Avg/Min	Avg/Max		
Back Lot	ж	2.0 fc	3.3 fc	1.4 fc	2.4:1	1.4:1	0.6:1		
rade	+	1.6 fc	28.7 fc	0.0 fc	N/A	N/A	0.1:1		
aorth Lot	ж	1.2 fc	3.8 fc	0.3 fc	12.7:1	4.0:1	0.3:1		
Property Line	+	0.1 fc	0.3 fc	0.0 fc	N/A	N/A	0.3:1		
Property Line ✓vest Lot	ж	2.0 fc	5.4 fc	0.4 fc	13.5:1	5.0:1	0.4:1		

S chedule											
Symbol	Label	QTY	Manufacturer	Catalog Number	Description	Number Lamps	Filename	Lumens per Lamp	LLF	Wattage	Lamp
-19-9457	Α	5	Lumenpulse	LIAM-120_277-CSL- M150-40K-CRI 80-3 BLS	Lumenicon Medium	1	LIAM-120_277-CSL- M150-40K-CRI 80-3 BLS.ies	7951	0.9	132	LED
DERS\105	В	5	Lumenpulse	LIAM-120_277-CSL- M150-40K-CRI 80-4 BLS	Lumenicon Medium	1	LIAM-120_277-CSL- M150-40K-CRI 80-4 BLS.ies	6519	0.9	132	LED
51 JOB FQL	С	2	Lumenpulse	LIAM-120_277-CSL- M150-40K-CRI 80-5S	Lumenicon Medium	1	LIAM-120_277-CSL- M150-40K-CRI 80- 5S.ies	13565	0.9	132	LED
K:\2019\10	D	30	Generation Brands	7000WTEG18UDWWC UNV830Y	LED Wall-Mount Luminaire	1	102171228CHI-046 GB 7000WTEG18UDWWC UNV830Y.ies	2420	0.9	30	LED

- **General Note** SEE SCHEDULE FOR LUMINAIRE MOUNTING HEIGHT.
- 2. SEE LUMINAIRE SCHEDULE FOR LIGHT LOSS FACTOR.

3. CALCULATIONS ARE SHOWN IN FOOTCANDLES AT: GRADE THE ENGINEER AND/OR ARCHITECT MUST DETERMINE APPLICABILITY OF THE LAYOUT

TO EXISTING / FUTURE FIELD CONDITIONS. THIS LIGHTING LAYOUT REPRESENTS ILLUMINATION LEVELS CALCULATED FROM LABORATORY DATA TAKEN UNDER CONTROLLED CONDITIONS IN ACCORDANCE WITH ILLUMINATING ENGINEERING SOCIETY APPROVED METHODS. ACTUAL PERFORMANCE OF ANY MANUFACTURER'S

₺7680.2

LUMINAIRE MAY VARY DUE TO VARIATION IN ELECTRICAL VOLTAGE, TOLERANCE IN LAMPS, AND OTHER VARIABLE FIELD CONDITIONS. MOUNTING HEIGHTS INDICATED ARE FROM GRADE AND/OR FLOOR UP.

THESE LIGHTING CALCULATIONS ARE NOT A SUBSTITUTE FOR INDEPENDENT ENGINEERING ANALYSIS OF LIGHTING SYSTEM SUITABILITY AND SAFETY. THE ENGINEER AND/OR ARCHITECT IS RESPONSIBLE TO REVIEW FOR MICHIGAN ENERGY CODE AND LIGHTING QUALITY COMPLIANCE.

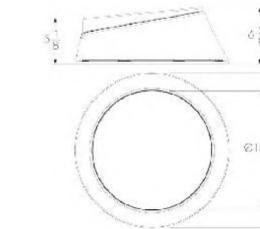
UNLESS EXEMPT, PROJECT MUST COMPLY WITH LIGHTING CONTROLS REQUIRMENTS DEFINED IN ASHRAE 90.1 2013. FOR SPECIFIC INFORMATION CONTACT GBA CONTROLS GROUP AT ASG@GASSERBUSH.COM OR 734-266-6705

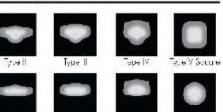
Specification Sheet

The Lumenicon Area Medium creates a consistent aesthefi while illuminating city streets, local roads, residential streets.

Side view







2200K 2700K 3000K 3500K 4000K 5700K

ON/OFF 0-10V

IP66 (optical champer)

Certifications

system makes the plug-and-play components easy to access. IP66 rated with phenomenal heat dissipation; the Lumenicon Area Medium is ready to take it to the streets (or parking lots, or building sides, or onramps, or...]. 2200K, 2700K, 3000K, 3500K, 4000K, 5700K Type II, Type III or Type IV (with or without backlight shield), Type 5 square and Type V Softsite 3G Vibration Rated Meets 3G ANSI C136,31-2010 vibration standard for bridge

protector, 5 pins and 7 pins receptacles with and without shorting cap 5-year limited warranty Performance Output (nominal lumens Minimum 3000lm / Maximum 20000lm

TM-21 L70 527,000 hrs (projected, Ta 77 °F), 36,000 hrs (reported Dark sky Dark sky compliant (2200K, 2700K and 3000K color temperatures. BUG rating of UO)

3 SDCM at CRI 70+ and 2 SDCM at CRI 80+

Die cast low copper 360 aluminum alloy Optical tempered clear glass (Clearsite lens), Optical tempered op al glass (Softsite Iens)

Lumen pulse Group Inc. reserves the right to make changes to this product at any time without prior notice and such modification shall be effective immediate 2020.02.26 copyright © 2020.02 menpulse Group Inc.

Color Rendering

TEGEL 18 WALL SCONCE

TECH LIGHTING

Understated modern design characterizes Tegel outdoor LED wall sconces. Providing well-controlled up and down lighting, or down light only, these wall sconces provide accent and ambient illumination. The option of three finish choices ensures this design profile successfully blends with all architectural aesthetics.

High quality LM80-tested LEDs for consistent long-life performance and color

Outstanding protection against the elements:

Powder coat finishes

 Impact-resistant, UV stabilized frosted acrylic lensing Up light and down light options, with 10° and 36° beam spread options

SPECIFICATIONS DELIVERED LUMENS 2419 Up-Downlight / 1212 Downlight 29:9 Up - Downlight / 15.6 Downlight WATTS Universal 120-277V, with integral transient 2.5kV VOLTAGE surge protection (driver) DIMMING 0-10, ELV LIGHT DISTRIBUTION Symmetric Up/Down Lighting, or Down Only MOUNTING OPTIONS OPTICS 10" and 36" PERFORMANCE OPTIONS Photocontrol / Surge Protector 2700K, 3000K or 4000K BUGRATING Up-Downlight B1-U5-G0/ Downlight B1-U0-G0 DARKSKY Compliant (Downlight) WET LISTED GENERAL LISTING Can be used to comply with CEC 2016 Title 24 Part 6 for outdoor use. Registration with CEC Appliance Database not required. START TEMP FIELD SERVICEABLE LED CONSTRUCTION HARDWARE Stainless Steel Powder Coat

L70; 70,000 Hours

5 Years



* Visit techlighting.com for specific warranty limitations and details.

ORDERING INFORMATION

LED LIFETIME

WARRANTY*

700 OWTEG CRI/CCT | 827 80 CR, 2700K | 18 IST | N | 10" NARKOW | C CLEAR | B BLACK | DO DOWNLIGHT CRLY | 830 80 CR, 3000K | W | 36" WIDE | Z BROKZE | UD UPLIGHT/DOWNLIGHT | 840 80 CR, 4000K | NN | 10" UP AND | H | CHARCOAL | Y GRAY NW 10" AND 36" "N, Ww.DOWNLIGHT ONLY (DO); NN, WW., NWw.UP AND DOWNLIGHT ONLY (LID

tech lighting.com

Designer Date 9/28/2020 Scale Not to Scale Drawing No. #20-48479-V5 PH-1

WIS