SITE PLANS FOR "NEAR NORTH" CITY OF ANN ARBOR, WASHTENAW COUNTY, MICI TAX ID# 09-09-20-412-041

GENERAL NOTES:

AND BREAK GROUND IN 2021

- 1. ALL CONSTRUCTION TO CONFORM AND COMPLY TO THE CURRENT STANDARDS AND SPECIFICATIONS OF CITY OF ANN ARBOR, WASHTENAW COUNTY ROAD COMMISSION, WASHTENAW COUNTY DRAIN
- COMMISSIONER AND ANY OTHER GOVERNING AGENCY. 2. LOADING/UNLOADING TO BE COMPLETED FROM IN FRONT OF THE TRASH COMPACTOR AREA.
- 3. WASTE WATER DISPOSAL TO BE DISCHARGED TO EXISTING CITY SEWER WITH ANN ARBOR, COUNTY, MDEQ APPROVAL.
- 4. WATER SUPPLY TO BE CONNECTED TO EXISTING CITY PUBLIC WATER SUPPLY 5. A PERMIT FROM MDOT IS REQUIRED FOR ALL WORK WITHIN N. MAIN STREET ROAD.
- 6. A SOIL EROSION AND SEDIMENTATION PERMIT FROM THE CITY OF ANN ARBOR WILL BE REQUIRED. 7. EXTERIOR LIGHTING TO BE WALL AND POLE MOUNTED AND SHIELDED PER CITY
- REOUIREMENTS. 8. SIGN DETAILS ARE TO BE PROVIDED TO THE CITY AND APPROVAL GRANTED PRIOR TO THE
- PLACEMENT OF ANY SIGNS. SIGN PLAN TO BE SUBMITTED AT A LATER DATE 9. SEE LANDSCAPE PLAN FOR PROPOSED LANDSCAPING
- 10. STORM SEWER DETENTION TO BE PROVIDED ON SITE IN DETENTION POND. ALL STORM WATER MUST BE APPROVED BY WCWRC AND ANN ARBOR.
- 11. ALL REOUIREMENTS BY THE FIRE DEPARTMENT PER THE INTERNATIONAL FIRE CODE WILL BE MET. 12. THE ANTICIPATED DEVELOPMENT SCHEDULE IS TO RECEIVE APPROVALS FOR CONSTRUCTION IN 2020

PROPRIETOR/DEVELOPER: MR. ANTHONY RANDAZZO TROWBRIDGE COMPANIES 2617 BEACON HILLS DRIVE AUBURN HILLS, MI 48326

SURVEYOR:

248-373-2440

TOPOGRAPHIC/BOUNADRY SURVEY MONUMENT ENGINEERING GROUP ASSOCIATES 638 SOUTH GRAND AVE FOWLERVILLE MI, 48836

MR. WILLIAM JARRATT JARRATT ARCHITECTURE WWW.JARRATTARCHITECTURE.COM 248-568-5200

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MR. MICH POWELL 4700 CORN

ARCHITECT:

WHITE LA

I) DEVELOPMENT PROGRAM

- a) <u>Description</u>: The site had been previously approved for development as an affordable housing rental project with 39 apartment units, retail and office space.
- i) The prior applicant rezoned the property to a PUD designation, but did not obtain final approvals and permits from Ann Arbor.
- ii) Five of the seven original single-family homes had been demolished and removed from the site.
- iii) The remaining two existing homes were demolished and removed in spring of 2016 by the present applicant.
- iv) The proposed development seeks the following approvals:
- (1) Removal of the Planned Unit Development designation, back to the underlying R4C multi-family zoning.
- (2) Reduction in the front yard setback from 25 feet to 10 feet following the average neighboring properties existing setbacks. Reduction of the rear requirement from 33 feet to 22 feet in the southeast corner of the site. (3) Site plan approval
- (4) Floodplain fill and mitigation for an area on the North side of the site. The existing floodway on the north end of the site will not be affected.
- v) The development program will be as follows:
- (1) 22 attached single family townhome units. (2) There two unit floor plans: The Narrower unit will have a footprint of about 16' x
- 40' on the tandem two car garage level, 17.5' x 42' on the upper levels and will be about 1,500 SF finish space. The Wider unit will have a footprint of about 17.5' x 40' on the two car garage level, 17.5' x 42' on the upper levels and will be about 1,700 SF finish space.
- (3) Units will have attached garages and either finished space at the kitchen living and dining on the main level, 2 bedrooms on the 2nd level, and a loft bedroom on the third level. The third level will be contained within the truss space, so the building will have a 2 story appearance.
- (4) Each building will two or three enclosed covered parking spaces and a small visitor parking lot of 7 spaces will be provided at the north end of the property.

- (5) A common deck over the access drive serving the garages will be constructed, providing each unit an outdoor recreational space. The deck will provide screens to separate each unit's private deck space from the other units. Stormwater detention will be contained in a concrete structure, the top of which will contain a
- 1,100 square foot outdoor recreation deck. b) Preliminary Phasing Proposal & Probable Construction Costs
- i) Three phases of construction are proposed. ii) Probable construction cost for site work, utilities and infrastructure is estimated to be

\$600,000. Vertical construction costs are estimated to be \$5,450,000. II) COMMUNITY ANALYSIS

- a) Impact of Proposed Development on Area Schools
- i) Increased tax base.
- ii) The anticipated number of school-age children from the development is estimated as follows: 16 units with 48 bedrooms = 38 children's bedrooms 38 bedrooms w/1 $\frac{1}{2}$ children/bedroom = 57 children
- b) <u>Relationship of Intended Use to Neighboring Uses</u>: To the south are single family homes with R4C zoning, to the west are the Ann Arbor Community Center and a former City public works building zoned PL and C1, to the north are Summit Party Store and two single family homes, zoned C1 and O, respectively, to the east are single family homes zoned R4C. So the area is characterized by a mixture of single family, institutional and retail uses. At a density of 18.6 units to the acre of attached single family units, the proposed
- development will provide a moderate increase in density in the area. c) Impact of Adjacent Uses on the Proposed Development: The adjacent uses are compatible. The Proposed townhomes are similar to the detached single family houses to the east and will provide a sound barrier for these homes from Main Street traffic. No negative impacts are expected to other surrounding uses.
- d) Impact of Proposed Development on Air and Water Quality and on the Existing Natural Features of the Site and Neighboring Sites:
- i) There will be no negative impact on air quality.
- ii) Water quality controls will be implemented to ensure that runoff during construction and from the additional impervious areas is managed and the current WRC stormwater

CONSTRUCTION NOTES:

- i. THE CONSTRUCTION COVERED BY THESE PLANS SHALL CONFORM TO THE CITY OF ANN ARBOR PUBLIC SERVICES STANDARD SPECIFICATIONS.
- ii. THE OMISSION OF ANY STANDARD DETAILS DOES NOT RELIEVE THE CONTRACTORS OF THEIR OBLIGATION TO CONSTRUCT ITEMS IN COMPLETE ACCORDANCE WITH THE PUBLIC SERVICES STANDARD SPECIFICATIONS. iii. USE OF LINE STOPS IS REQUIRED WHERE EXISTING WATER MAINS CANNOT BE SUFFICIENTLY ISOLATED TO COMPLETE THE WORK. THE COST OF ANY LINE STOP INSTALLATION IS THE RESPONSIBILITY OF THE DEVELOPER AND/OR CONTRACTOR. iv. PAVEMENT MARKINGS DISTURBED DUE TO PAVEMENT CUTS OR CONSTRUCTION RELATED ACTIVITIES SHALL BE REPLACED AS DIRECTED BY ENGINEERING. REPLACEMENT DURING CONSTRUCTION OF THE PROJECT MAY BE CONSIDERED TEMPORARY, WITH FINAL PAVEMENT MARKING RESTORATION TO OCCUR AT THE

END OF THE PROJECT. v. THE CONTRACTOR SHALL TAKE ALL NECESSARY PRECAUTIONS TO PROTECT THE EXISTING PUBLIC ROAD PAVEMENT. DAMAGE TO THE PUBLIC ROAD PAVEMENT DURING THE COURSE OF CONSTRUCTION MAY NECESSITATE MILLING AND RESURFACING OF THE DAMAGED AREAS PRIOR TO ISSUANCE OF THE CERTIFICATE OF OCCUPANCY.

| | | | (| | | |
|--|-------------------------------|--|--------------------------------|--|--|---|
| | Zoning: | Parcel 1 | - | Existing PUD | Required R4-C | Proposed R4-C |
| | Existing [| Owelling Units | | none Approved PUD | R4C req | Proposed |
| | Proposed | I Dwelling Units | | 39 Units | 23 units max | 22 Units |
| | | | | 2,714 sf retail | | |
| HIGAN | | | | 1,553 sf office | | |
| | Minimum Minimum Maximum | Lot Width | | 51,444 sf 362 ft 1,329 sf/d.u. | 8.500 sf 60 ft 2175 sf/d.u. | 51,444 sf 362 ft 2338 sf/d.u. |
| | Minimum Minimum | Useable Open Space Active Open Space Vehicle Parking Spaces per Dwelling | aces | 33 units/ac 59% not required 53 1.35 sp/unit | 20 units/ac 40% 300 sf/unit 26 1.5 sp/unit | 18.6 units/ac 43% 385 sf/unit***** 55 2.5 sp/unit |
| | Bicycle Pa | | | 39 spaces | 4 spaces 50% class A 50% class C | 4 spaces 22 class A 4 class C |
| INEER: NGINEERING CHAEL POWELL L ENGINEERING & ASSOCIATES RNERSTONE DRIVE LAKE, MI, 48383 | Setbacks | Minimum Maximum | Front Front Side Rear | 15' 15' 13.5' 24' | 10'* none 19'3''***** 47'3''** | 10' 10' 15' 22' minimum one corner 27.5' average |
| | Height | | | 50' | 30' | 30' |
| | - | ide Depth Dimensic ront Width Dimensi | | | | 108 ft 188 ft |

* average front setbacks w/in 100 ft 8.67' ** incl addit setback for 188' bldg length *** incl elevated common deck as open space ***** bldg is 108' long on side & shorter than 35' on south end of site ****** incl patio and elevated decks

LEGAL DESCRIPTION

Schedule A Description:

Situated in the County of Washtenaw, State of MI, is described as follows:

Commencing at the monumented intersection of Main Street and Summit Street; thence S 62°00'42" E 37.46 feet to a point along the centerline of Summit Street, said point being N 62°00'42" W 366.67 feet from the monumented intersection of Summit Street and N. Fourth Street; thence S 19°00'00" W 114.21 feet parallel to the monumented centerline of Main Street and along the westerly line of Lot 1 of Block 1, Ormsby and Page's Addition, as recorded in Liber M of Deeds, Pages 191 and 192, Washtenaw County Records, Michigan for a Point of Beginning;

thence S 71°02'44" E 80.66 feet; thence S 18°20'18" W 29.33 feet;

- thence S 70°52'52" E 10.05 feet;
- thence S 28°13'07" W 1.73 feet;

thence S 71°07'58" E 67.47 feet along the north line of Lot 5 of said Addition; thence S 22°03'45" W 208.08 feet along the east line of lots 5, 7, 9 and 11 of said Addition to a found iron pipe;

thence S 22°36'53" W 123.99 feet along the east line of lots 11 and 13 of said Addition; thence N 70°53'11" W 139.30 feet along the south line of Lot 13 of said Addition to a point which lines N 19°00'00" E 215.31 feet parallel to the monumented centerline of Main Street, along the easterly right-of-way line of Block 1 of said Addition from the southwesterly corner of Lot 19 of said Addition:

thence N 19°00'0" E 362.08 feet parallel to the monumented centerline of Main Street, along the easterly right-of-way line of Block 1 of said Addition to the Point of Beginning. Being a part of Lots 1 and 2 and all of Lots 5, 7, 9, 11 and 13 of Block 1, Ormsby and Page's Addition, in the SE 1/4 of Section 20, Town 2 South, Range 6 East, in the City of Ann Arbor, Washtenaw County, Michigan.

- regulations. There are no adjacent bodies of water into which stormwater discharge will flow
- iii) A total of 14 non-landmark trees will be removed (131 diameter inches) and 5 landmark trees (136 diameter inches) will be removed mitigated.
- iv) There are no regulated wetlands on the site.
- v) Storm sewers draining roofs & paved areas will be developed on site. The water and sanitary sewer connections will be via laterals to each building from the existing mains located in the Main Street right of way fronting the site.
- e) Existing Lane Use or Historic Sites/Structures: No historic structures exist on site. The site itself is not historic.

III) SITE ANALYSIS

- a) *Existing Land Use and Activity on the Site*: The site is vacant. The property is not currently being farmed.
- b) Inventory of Site Conditions: The site generally drains to the west and north. The site is generally open, being greatly disturbed through the removal of existing houses that once occupied the site. The site also contains several City regulated trees. See Tree Inventory There is one area of the site where an existing retaining wall creates a steep slope condition. The remainder of the site, while sloping, wouldn't be consider steep sloped.
- c) <u>Natural Features Descriptions</u>
- i) There is no known endangered species habitat on the site.
- ii) There is a 100 year floodplain and a floodway that encumbers the northern 1/3 of the site. (128 CY) of the 100 year floodplain will be filled for part of the driveway serving the site. Compensating cut will be provided on the north end of the site per City, State and Federal requirements.
- iii) There are no streams, rivers, lakes or bodies of water adjacent to or on site.
- iv) There are trees on the site including 5 landmark trees that are being removed as part of this development. These are being mitigated with replacement trees on site in accordance with City tree replacement requirements.
- v) 5 landmark trees in the area being disturbed are being removed. These are being mitigated with replacement trees planted on site in accordance with City tree replacement requirements.

BENCHMARKS

DATUM: NAVD88

ANN ARBOR GEODETIC REFERENCE SYSTEM (AAGRS) REFERENCE BM'S (CITY OF ANN ARBOR GEODETIC CONTROL MANUAL - 2004)

- AAGRS/NGS NO. 0014A:
- ELEV = 786.13
- AAGRS/NGS NO. 0013A ELEV = 829.12
- AAGRS NO. 1010
- ELEV = 811.95

SITE BM'S:

BM A:

BIG ARM HYDRANT 34'± WEST NORTH MAIN STREET, 157'± SOUTH FROM INTERSECTION NORTH MAIN STREET AND EAST SUMMIT STREET. ELEV = 778.56

BM B:

NAIL ON NORTH SIDE UTILITY POLE, 48'± NORTH FROM SUBJECT'S SOUTHWEST PROPERTY CORNER, 26'± EAST NORTH MAIN STREET. ELEV = 788.93

BM C:

NAIL ON SOUTH SIDE UTILITY POLE, 190'± NORTH FROM SUBJECT'S SOUTHWEST PROPERTY CORNER, 26'± EAST NORTH MAIN STREET. ELEV = 782.11

- vi) Generally, the tree protection fence will be placed at the edge of the critical root. In some cases, this is not possible and still save the tree; here, the tree protection fence will not be placed closer than 10 feet to the trunk of the tree. If placed closer than 10 feet from the trunk it shall be mitigated for.
- d) *Existing Structures*: None.
- e) Existing Access Points
- i) Existing vehicle driveway cuts to Main Street will be closed. New internal private drives will intersect Main Street at one point; said location being preliminarily approved by MDOT.
- ii) Pedestrian access will include: sidewalks along Main Street and lead walks serving unit front entries that face east.
- f) <u>Utilities</u>
- i) A 14 inch water main is located within the Main Street right of way and will be tapped near the proposed road entry to the site to install a fire hydrant and provide fire
- suppression and domestic water laterals to each building. ii) An 24 inch sanitary sewer is located within the Main Street right of way. 6 inch sanitary laterals are proposed to each building.
- iii) Gas, electric, telephone and cable will be extended into the site from their present locations along Main Street.
- g) <u>Drainage Patterns</u>: Presently, the site drains to the north and west toward Main Street.
- h) *Existing Land Use*: The property is presently vacant.

IV) SCHEMATIC DESIGN

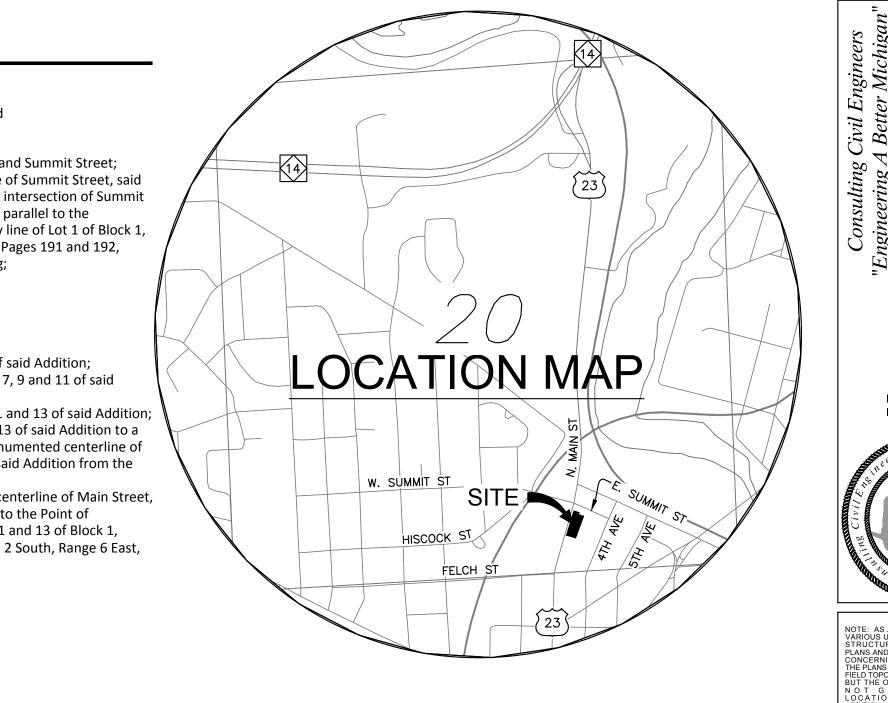
- a) A comparison chart of the proposed development and the City regulations is shown on the Sheet S2
- c) Orientation and general location of proposed improvements are shown on the Sheet S2.
- d) There are no areas of wetlands on the site and therefore no natural features buffers required.
- f) Existing landmark and City regulated trees are shown on the Sheet L-1. Tree mitigation replacement plantings are shown on the Sheets L-2 and L-3.

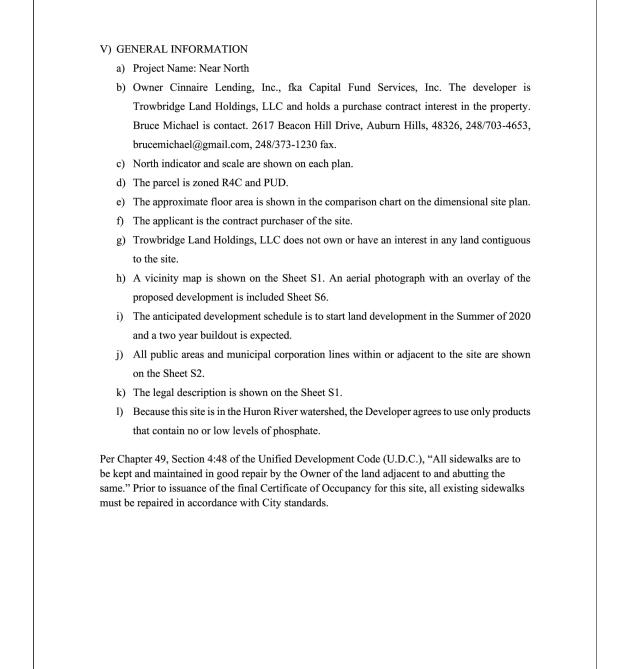
SHEET INDEX

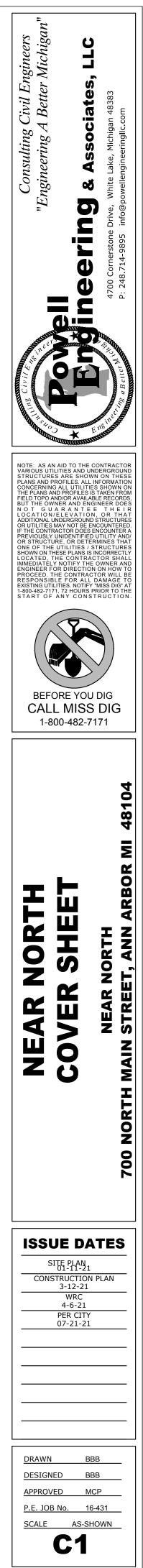
C1) COVER SHEET C2) EXISTING CONDITIONS PLAN C3) DIMENSIONAL LAYOUT PLAN C4) NATURAL FEATURES OVERLAY & SOIL EROSION PLAN C5) UTILITY PLAN & DRAINAGE ANALYSIS C5.1) AREA 2 DRAINAGE ANALYSIS C5.2) NORTH STORM PLAN DETAIL C5.3) PROFILES & DETAILS C6) GRADING C7) FLOODPLAIN PLAN **C8) CONSTRUCTION PHASING PLAN**

C9) DEMOLITION PLAN

b) The existing and proposed topography and limits of soil disturbance are shown on the Sheet e) Proposed buildings and setbacks are shown on the Sheet S2.



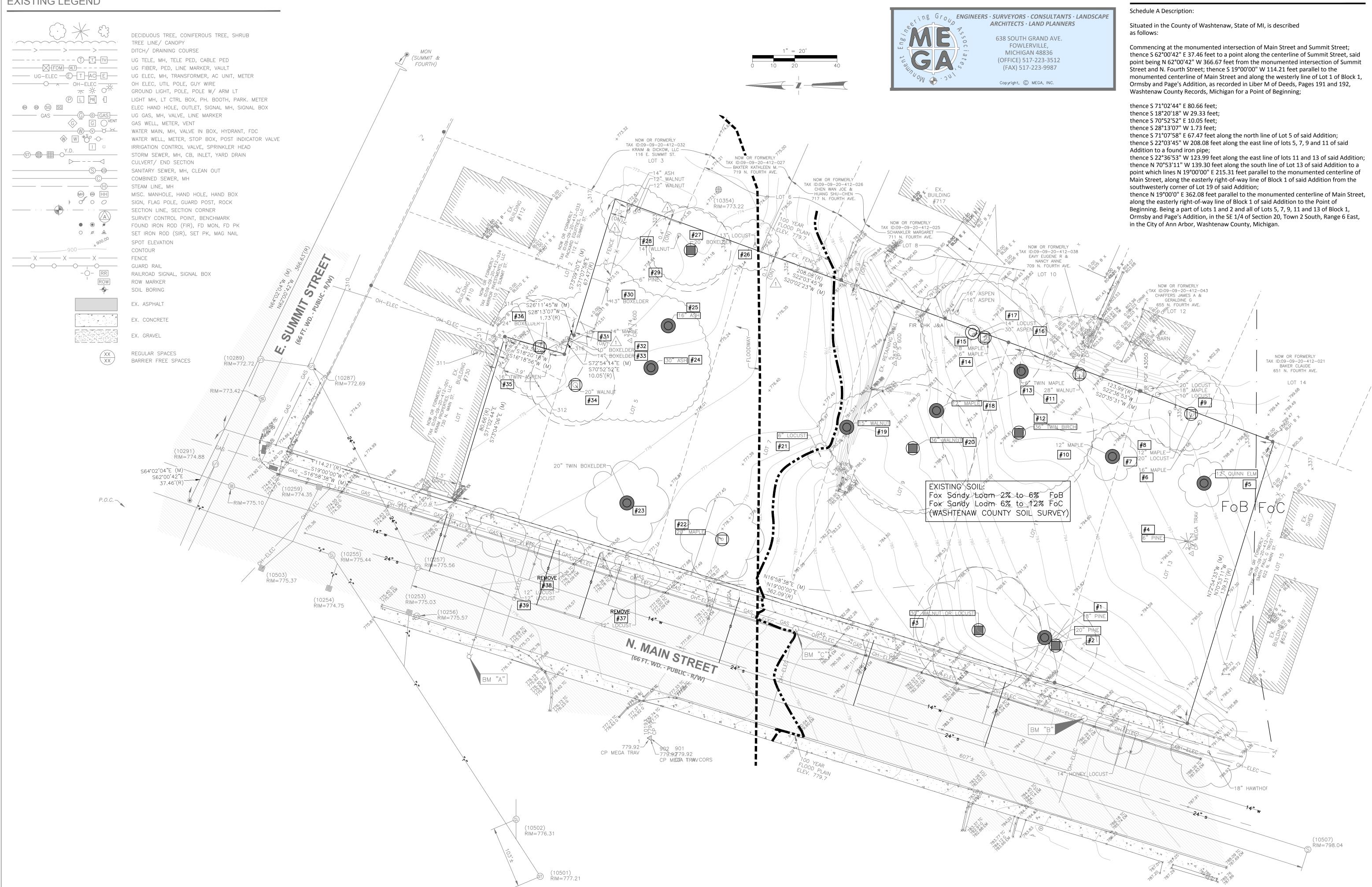




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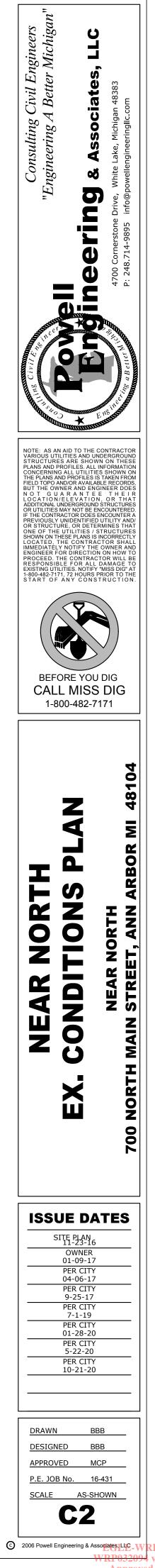
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EXISTING LEGEND

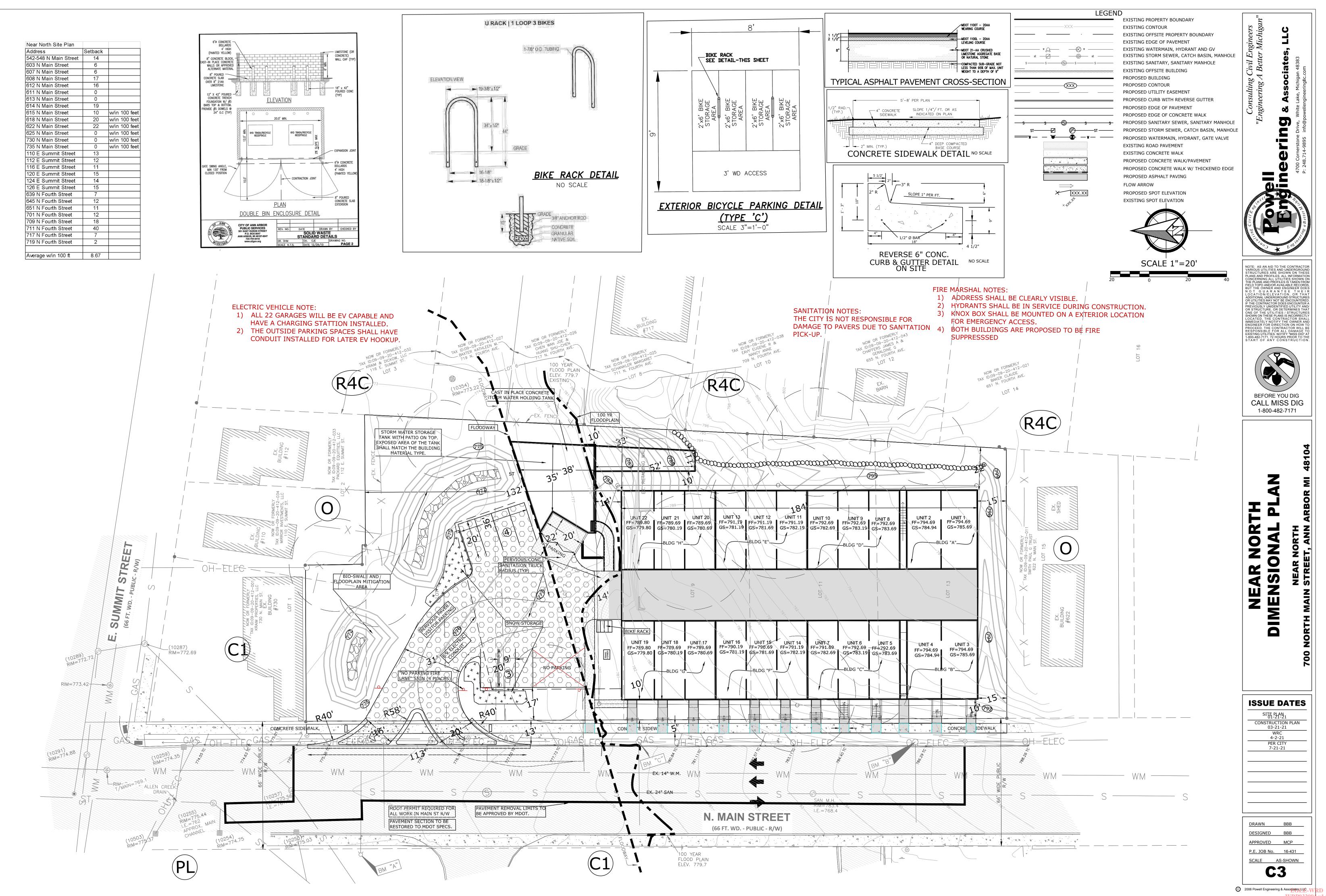


ALTA SURVEY BY:

LEGAL DESCRIPTION

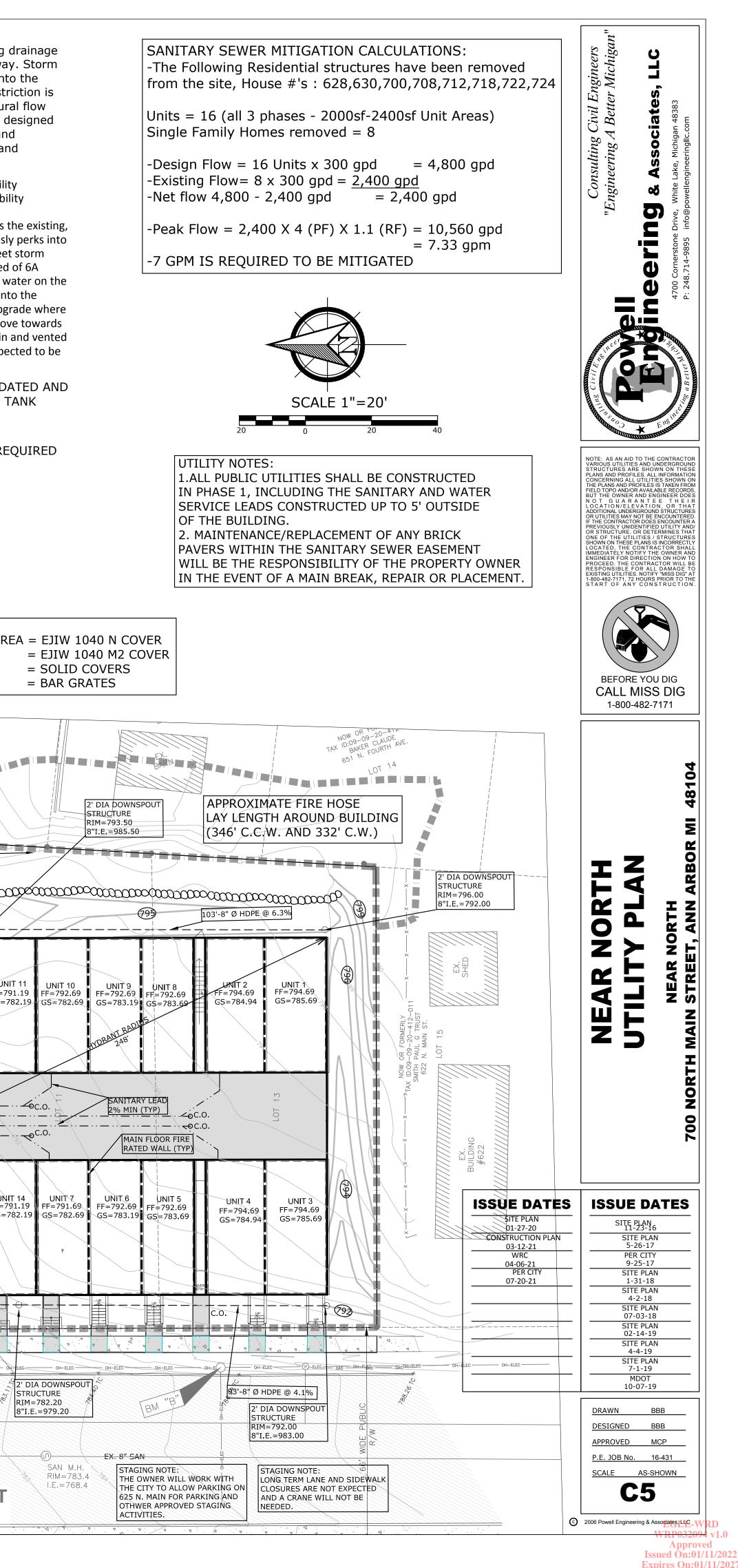


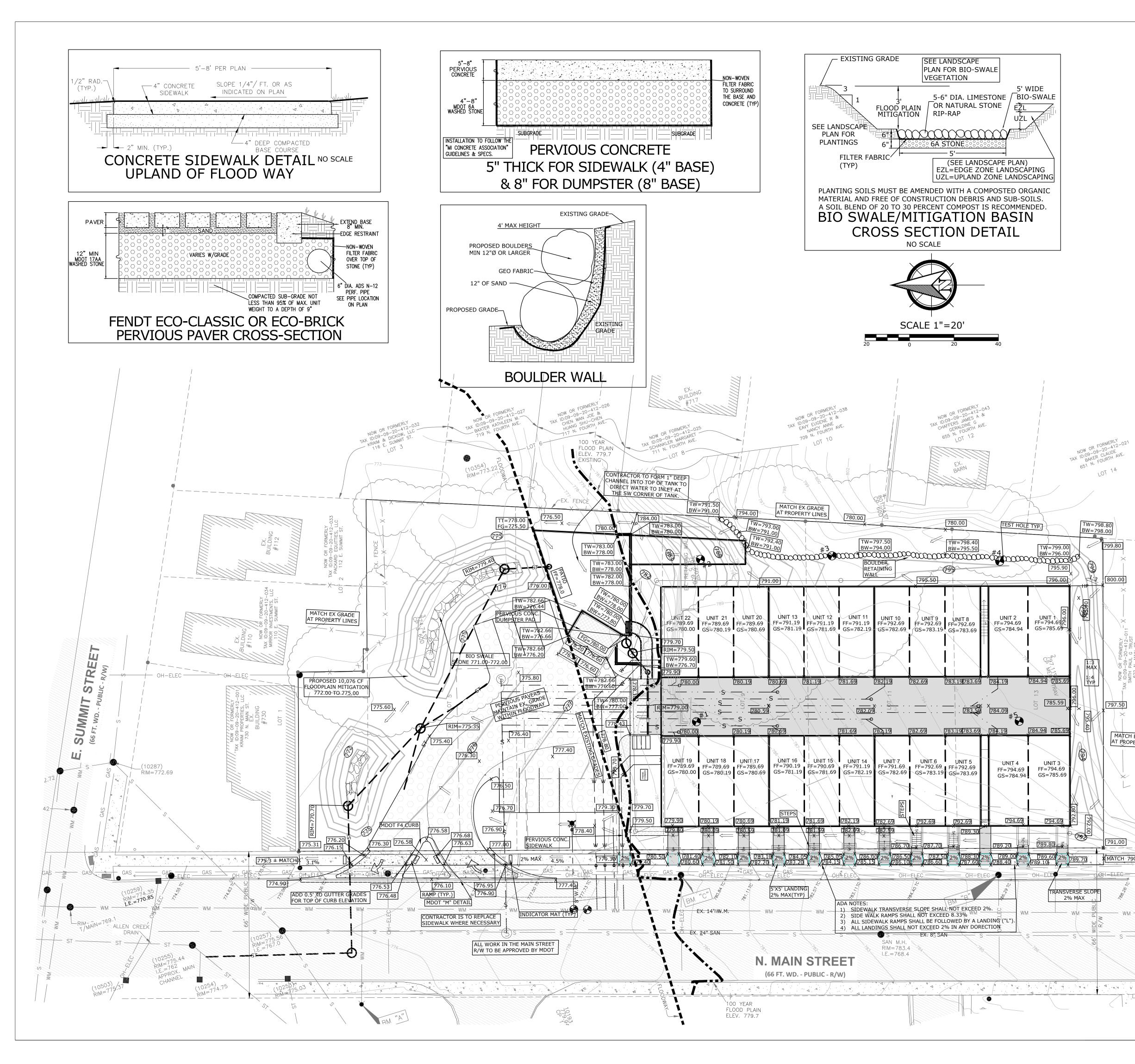
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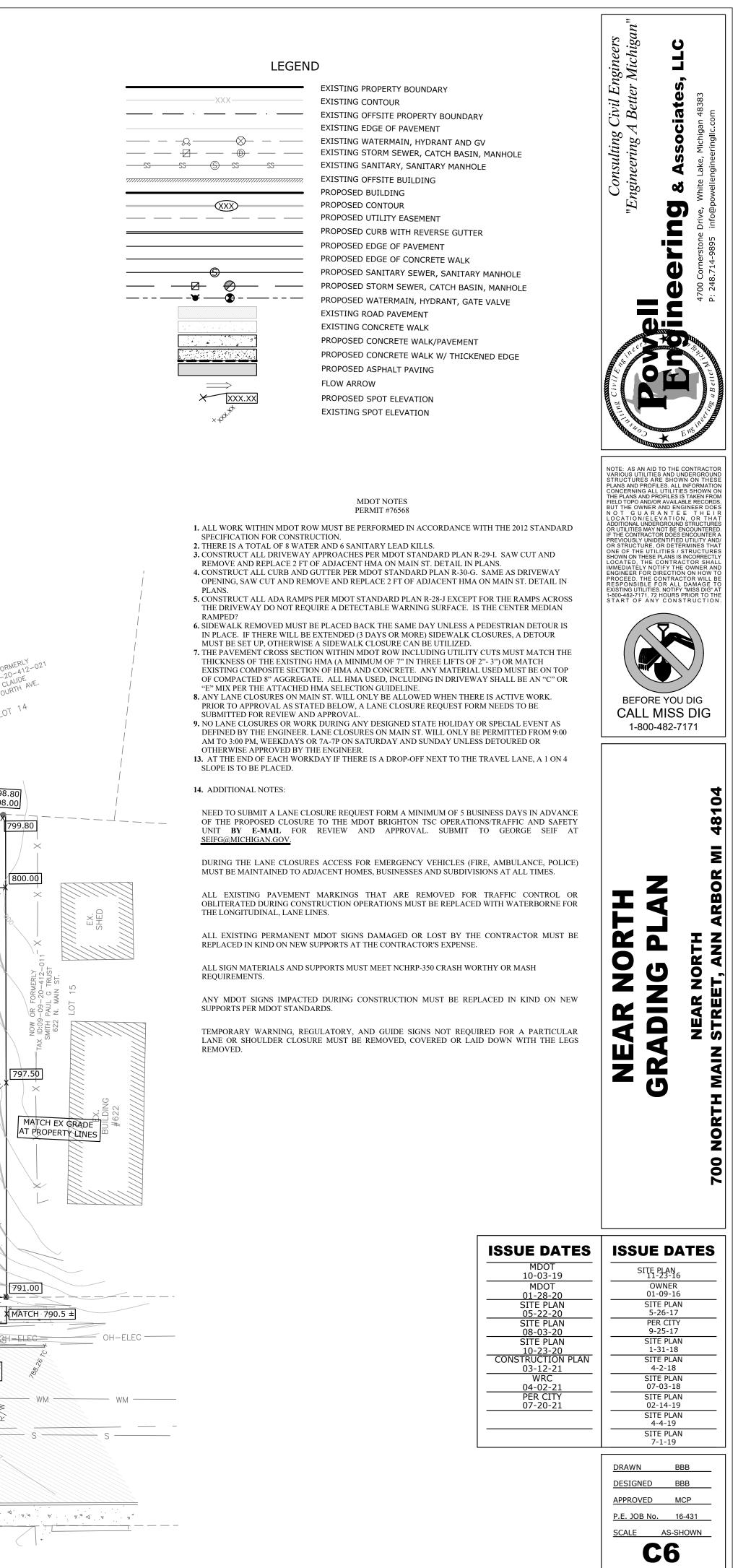


Approved Issued On:01/11/2022

| W1-C FACTORS AND CURVE NUMBERS | | | W7 - STANDARD METHOD RUNOFF VOLUME CA | ALCULATIONS | | | W10 - STANDARD METHOD RUNOFF VOLUME CALCULAT | 10NS V100= | = 11,965 cf | DRAINAGE NARRATIVE: |
|--|------------------------|---------------------|---|---------------------------------------|---|--|--|-------------------------------------|---|--|
| Comp "C" 0.49 ac. 0.95 0.46 | | | "Impervious Cover" Post Development 100-ye | ar Runoff Calcula | tions (V100-imp-pos | | Detention/Retention Requirement | | = 11,965 cf | Onsite storm water management will be accomplished by storing dra area A1 runoff in an onsite storage tank upland from the Floodway. |
| perv 0.25 ac. 0.30 <u>0.08</u> 0.54/ 0.7 | 4 = 0.73 | | A. | | | | A. Qp=238.6 Tc^(-0.82) Tc= | 0.09 hr | 1719 cf | water will then be released from the tank by restricted orifices into the Bio-Swale and then to a structure at the road R/W. The tank restriction |
| CN-imp 98 0.49 ac. CN-perv 61 0.25 ac. | | | 100-YR STORM EVENT | P= | 5.11 in | | peak of the unit hydrograph | | | designed to maintain the overall site pre-developement agricultural fl |
| W2 - STANDARD METHOD RUNOFF VOLUME CA | | | B. IMPERVIOUS COVER CURVE NO.(FR: | WS1) CN= | 98 | | B. Total Site Area (ac) excluding "Self Crediting" | BMP's | 0.74 AC | rate as it enters the public storm drain. The Bio-Swale has been design to reduce the velocity of the water that is outlet from the tank and |
| FIRST FLUSH VOLUME CALCULATIONS (Vff) | | | C | | | | C. Q100=A100-per+Q100-imp | Q100= | 5.09 in | provide evapo-transpiration and treatment in the stone/rip-rap and plantings. |
| TRIBUTARY A 0.74 ac | | | S = 1000/CN-10 | S= | 0.20 in | | D. Peak Flow (PF) = Qp(cfs/in-mi^2) *Q100(in) *A | Area(ac) / 640 PF= | 10.11 cfs | North Half of property is FoB Fox Sandy Loam 2-6% - Moderate Permeability South Half of property is FoC Fox Sandy Loam 6-12% - Moderate Permeability |
| Runoff Coeff 0.73 | | | D. Q = (P-0.2S)^2 / (P+0.85) | Q= | 4.87 in | | E. DELTA = PF (cfs) - 0.15 *Area(ac) F. Vdet=DELTA(cfs)/PF *V100(cf) | DELTA= | 10.00 cfs | Currently, during a rainfall event, the runoff produced sheet flows across the |
| Vff=(1")(1/12")(43560sf / 1ac) *A * 1961 cf W3 - STANDARD METHOD RUNOFF VOLUME CA | LCULATIONS | | E. IMPERVIOUS COVER AREA (Fr:WS1) | | | | W11 - STANDARD METHOD RUNOFF VOLUME CALCULAT | | | grassy surface (varying slope 3% - 10%). Some of the initial runoff obviously pe the Silty topsoil layer, but the balance travels overland into the main street sto |
| Pre-Development Bankfull Runoff Calculations | (Vbf-pre) | | | AREA= AREA= | 0.49 ac 21344.4 sf | | No infiltration can be provided so no credits are available | | | sewer system. The pervious brick pavers will be constructed on a level bed of 6 washed stone, very similar to the system at the Ann Arbor City Hall. Rain wate |
| A. | | | F | | | | W13 - SUMMARY | | | pavers will drop between the pavers into the voids of the 6A stone and onto the silty/sandy/clay subgrade. The water under the pavers will set on the subgrade |
| 2-YR/24 HOUR STORM EVENT | P= | 2.35 in | V100-imp-post = Q(1/12)Area V100 W8 - STANDARE METHOD Runoff Volume Cal | | 8668 ac | | A. STORMWATER MANAGEMENT SUMMARY | | | some will perk away (as in the original condition) and some will slowly move to the north end of the pavers and be picked up in the perforated edge drain and |
| B. See rules for requirement "Good cover woods or Meadow" | CN= | 61 | | | | | Min imum Onsite Infiltration Requirement, Vinf | | 3634 cf | out into the bio swale system. The runoff from the brick paver area is expected less that the existing turf surface. |
| C. | | | Flow Type K Ch. in Elev. Length (L) Sheet Flow 0.48 25 380 | 6.58 6.58 | 5) S^0.5 V=K*S(2.56 1.23 | 0.5 Tc=L/(V*3600) 0.09 | Designed/Provided Infiltration Volume | | 0 cf | OFFSITE DRAINAGE AREA A3 = 0.12 AC. WILL BE ACCOMMODATE |
| S = 1000/CN - 10 | S= | 6.39 in | | | Тс | 0.09 | Total Calc % Minimum Infiltration Provided | | 0 % | PASSED THROUGH OVERFLOW STRUCTURE IN THE STORAGE TAN PERVIOUS PAVER STONE VOLUME |
| D. Q = (P-0.2S)^2 / (P+0.8S) | Q= | 0.15 in | A. Runoff Summary from Previous Worksheets | | | | Net Required Dention Volume, (Vdet - Provided Infiltrat | tion Volume) | 11,834 cf | 5228 sf X 1 FT. = 5228 CF X 30% = 1568 cf SUPPLIED 100 YR STORM VOLUME = 16.500(0.7)(0.12 AC) = 1,386 cf REQU |
| E. | | | WS2 : First Flush Volume (Vff) | | | 1961 cf | B. DETENTION VOLUME INCREASE FOR SITES WHERE THE | REQUIRED INFILTRATION \ | 11,834 cf | 1,568 cf > 1,386 cf |
| TOTAL SITE AREA EXCLUDING SELF-CR | EDITING BMP'S AREA= | 0.74 ac | WS3 : Pre-Development Bankfull Ru | noff Volume (Vb | f-pre) | 413 cf | % Required Infiltration NOT provided | | 100 % | Mitigation Volume Supplied - 772.00 - 775.00. 775.00 4943 4398 1.0 10127 Mitigation supplied |
| | AREA= | 32234 sf | WS4 : Pervious Cover Post-Dev. Banl | cfull Volume (Vb | f-per-post) | 273 cf | Net % Penelty | | 20 % | 774.00 3852 3388 1.0 5729 773.00 2823 2341 1.0 2341 |
| F. Vbf-pre = Q(1/12) Area | Vbf-pre= | 413 cf | WS5 : Impervious Cover Post-Dev. B | ankfull Volume (\ | Vbf-imp-post) | 3774 cf | Total Required Detention Volume, including penalty (100% + Net % Penalty) x Net Required Detention Volum | ne) | 14,200 cf | 772.00 1858 0 0 Bottom Bio-Swale Mitigation Volume Supplied = 10,127 > 7000 cf required |
| W4 - STANDARD METHOD RUNOFF VOLUME CA | | | Total Post Development Bank Full V | olume Required | Vbf= | 4047 cf | TOTAL DETENTION VOLUME REQUIRED | | | |
| | | ()//hf rear reart) | WS6 : Pervious Cover Post-Dev. 100- | year Volume (V1 | 00-per-post) | 1303 cf | (A1=14,200 CF) + (A2 = 2783 CF) = 16 | 5,983 CF | | |
| Pervious Cover Post Development Bankfull Run | on Calculation | (vbi-per-post) | WS7 : Impervious Cover Post-Dev. 10 | 00-yr Volume (V1 | .00-imp-post) | 8668 cf | STORAGE TANK VOLUME SUPPLIED 766.9 - 775.9.00 = 9 ft depth | | | STRUCTURE SCHEDULE: CATCH BASINS IN LAWN AREA |
| A. 2-YR/24 HOUR STORM EVENT | P= | 2.35 in | Total Post Development 100-yr Volu | | vbf= ration = 120%V = | 9971 cf 11965 cf | 1986 sf/ft x 9 ft = 17874 cf 17,874 cf > 16,983 cf | | | CATCHBASINS IN CURBS MANHOLES |
| B. Dentions Course From W/S1 | CN | | B. Determine Onsite Infiltration Requirement Subtract Pre-Development Bankfull | | | | | | | END SECTION |
| Pervious Cover From WS1 | CN= | 61 | | | | 4047 cf | NOW OR FORMA 12-00 NO:09-09-20-412-00 TAX 10:09-09-CKOW, LLC TAX 00.08 & CUMMIT ST. | BAXIE FOUN | 717 N. 100 Y | NOW ON 9-20-CAREN TO TO TO |
| C. S = 1000/CN - 10 | S= | 6.39 in | Total Post-Development Bankfull Vo | | | | KRAINE E. SUIT 3 | | FLOOD ELEV. EXISTII | 779.7 |
| D. | | | Pre-Development Bankfull Volume | (vbf-pre) | | 413 cf | THE EVENT OF A SEWER BREAK | SEWER TO BE PREMIUM | | |
| Q = (P-0.2S)^2 / (P+0.8S) | Q= | 0.15 in | Bankfull Volume Difference | | | 3634 cf | REPAIR, OR REPLACEMENT IS THE RESPONSIBILITY OF THE OWNER. | | | IOP OF INSIDE OF TANK = 776.9 2' DIA DOWNSPOUT TOP OF STANDPIPE = 775.90 STRUCTURE 2" DIA. PVC RESTRUCTION BIM=782.50 |
| E. Pervious Cover Area from WS1 | | | On-Site Infiltration Volume Require | | V(inf)= | 3634 cf | 19'-12" | Ø RCP @ 1 0% | EX. FENCE | IBO1/IANK & NW I.E. = 766.90 NW 8"1.E. = 7780.20 S 8"I.E. = 780.20 |
| | AREA= AREA= | 0.49 ac 21344 sf | Compare the BF volume difference and the FF | | | | WAPRO "WASTOP" (OR EQUA BACK FLOW PREVENTOR | | | 794 DRAINAGE AREA: |
| F. | | | Therefore, due to poor soils infiltration will no | t be possible and | we will store 120% of | | BUNNET WW I.E. = 766.70 WW I.E. = 766.70 | | 34'-8" Ø HDPE @ 4' DIA CB W/ | $\frac{ AREA A1 = 0.74 \text{ AC.}}{ FLAT }$ |
| Vbf-per-post = Q(1/12) Area Vbf-p W5 - STANDARD METHOD RUNOFF VOLUME CA | • | 273 cf | | | | | BIO-SWALE | | TOP & LOCKE RIM=779.50 SE8"I.E.=77 | 6.10 |
| Impervious Cover Post Development Bankfull R | unogf Calculati | on (Vbf-imp-post) | | | | | AREA: 0.45 AC | 613 | W8"I.E.776.: SW12"I.E.=7 | |
| A. | | | | | | -412-03- -412-03- T ST. LL03- T ST. | <u>, vita AC.P</u> | 2' DIA. ACCESS MH RIM | ADS BARRAC RIM=779.60 SW12"I.E.=7 | |
| 2-YR/24 HOUR STORM EVENT | P= | 2.35 in | | | | M FORM 09-20- IVESTMEN SUMMI | | RIM=778:00 | W12" I.E.=7 | UNIT 22 UNIT 21 UNIT 20 UNIT 13 UNIT 12 UNIT 11 FF=789.80 FF=789.69 FF=789.69 FF=791.19 FF=791.19 FF=791.19 |
| B. Impervious Cover From WS1 | CN= | 98 | | | EX. LDING | NOW C RROR ID:09- 110 E | | PERVIQUS CONC. | 1980 1980 | GS=779.80 GS=780.19 GS=780.69 GS=781.19 GS=781.69 GS=782.19 10'-12" Ø HCPE @ 4.0% |
| C. | | | | | # CI | AT A | SAU | M=775.90 \ 🗌 SHAL | TION SYSTEM | ELBOW |
| S = 1000/CN - 10 | S= | 0.20 in | OH-ELEC- | OH-ELEC | - OH-ELEC OH-ELEC | - OH-ELEC | × 82'-12" Ø N-12 @ 1.0% | | SANITARY MENT | RIM=779.00 12"I.E.=777.80 MAIN FLOOR FIRE |
| D. Q = (P-0.2S)^2 / (P+0.8S) | Q= | 2.12 in | BLC. | | -001 | | | | ERVIOUS CONC. — SIDEWALK — — — | RATED WALL (TYP) C.O. |
| E. | | | | | 777777 DRMERLY 20-412- 212-412- AIN ST. | 30 L 1 30 | 3'-12" Ø N-12 @ 1.0% | 0 FT WIDE | | |
| Impervious Cover Area from WS1 | AREA= | 0.49 ac | CUM Birt. WI | | W OR F W OR F 09-09- 30 N. M | ++////##1 | | SL8 - 12" DIA | | |
| | AREA= | 21344.4 sf | | | | 1.0 | | RIM=775 | | 180'-8" Ø HDIE @ 0.5% FDC |
| F. Vbf-imp-post = Q(1/12) Area Vbf-in | np-post= | 3774 cf | | 10287) IM=772.69 | RIM=7 | 770.80 I.E.=765.40 I.E.=765.40 | | | ROPOSED "Ø FIRE SUPP. VATER | UNIT 19 FF=789.69 FF=789.69 FF=789.69 FF=789.69 FF=780.19 FF=790.19 FF=790.69 FF=790.69 FF=790.69 FF=791.19 GS=781.69 GS=781.69 GS=781.69 GS=781.69 GS=782.19 |
| W6 - STANDARD METHOD RUNOFF VOLUME CA | | | | IM=//2.09 | | <u>1.L.=705.40</u> | | | ERVICE V ROPOSED "Ø DOMESTIC | |
| "Pervious Cover" Post-Development 100-YR Rur | off Calculation | IS | | | | | | | ATER ERVICE | 2' DIA DOWN SPOUT STRUCTURE |
| A. 100-YR STORM EVENT | P= | 5.11 in | | | | | PERVIOUS PAVERS MAINTAIN EX. GRADE WITHIN FLOODWAY | | T W.M. | STRUCTORE RIM=781,00 8"I.E.=776,50 |
| B. | | | DN ERATED | | 66'-12" @ | 0 RCP @ 2:5% | | 0+3 PR.H | | |
| PERVIOUS COVER CURVE NO. (FR:WS1 | .) CN= | 61 | | · · · · · · · · · · · · · · · · · · · | do. d. b | | | | 0+33 CER 6">8" | W W W W W W W W W W W W W W W W W W W |
| C. S = 1000/CN - 10 | S= | 6.39 in | | | ELEC OH ELEC OH | CAS | S CAS CH-ELEC CH-E | CAS CAS CAS CAS CAS CAS CAS CAS CAS | GAS - | and the second s |
| D. | | | (10259) RIM=774 | 4.35 44 S | PUBLIC | 200°,C | % CRCSSING #2 % TOP/ST=765.62 BOT/WM=770.00 ↓ | Xe I ISTA. | D+00 16 TEE | ALL EXISTING CURB CUTS SHALL BE RESTORED PER MDOT STNDS |
| D. Q = (P-0.2S)^2/ / (P+0.8S) | Q= | 1.44 in | STRUCUKE BE VERIFIED RIM- | | | CROSSING TOP/ST=7 | 65.22 | EX. 14" W.M | | EX. 14" W.M. |
| | | | RIM-TOP: 769.11 T/MAIN=769.11 ALLEN CREE DRAIN | 4 12"Ø R K TO WR | CP ACCORDNG C STANDARDS =763.00 | BOT/SAN= | 767.12 | P. EX. 24" SAN | | VELL 16" TO 14" REDUCERS EX. 24" SAN |
| PERVIOUS COVER AREA (Fr: WS1) | AREA= | 0.25 ac | | | | 025175.56 IM=775.56 .E.=767.0 | 4' Ø CB "B" RIM=775.75 E I.E.=763.72 PR. SANITARY TA | | | |
| | AREA= | 10890 sf | | (10255) RIM=775.44 RIM=762 MAIN | TOP/S | SING #3 ST=764.53 | E 1.E. = 763.62 PR. SANITARY 1/ PR. RIM=777.30 66'-12" Ø RCP @ 0.94% EX. I.E.=767.52 | | 33 | 3 LF OF 14" C.I.P. TO BE REMOVED REPLACED WITH 33 LF OF 16" D.I.P. 2 16" DIP TO 14" CIP REDUCERS |
| F. V100-per-post = Q(1/12) Area V100- | per-post= | 1303 cf | 10503) ×1 | APPROA. CHANNEL CHANNEL | BOT/S | SAN=767.08 | | | | (66 FT. WD PUBLIC - R/W) |
| | | | | | | | | | | |

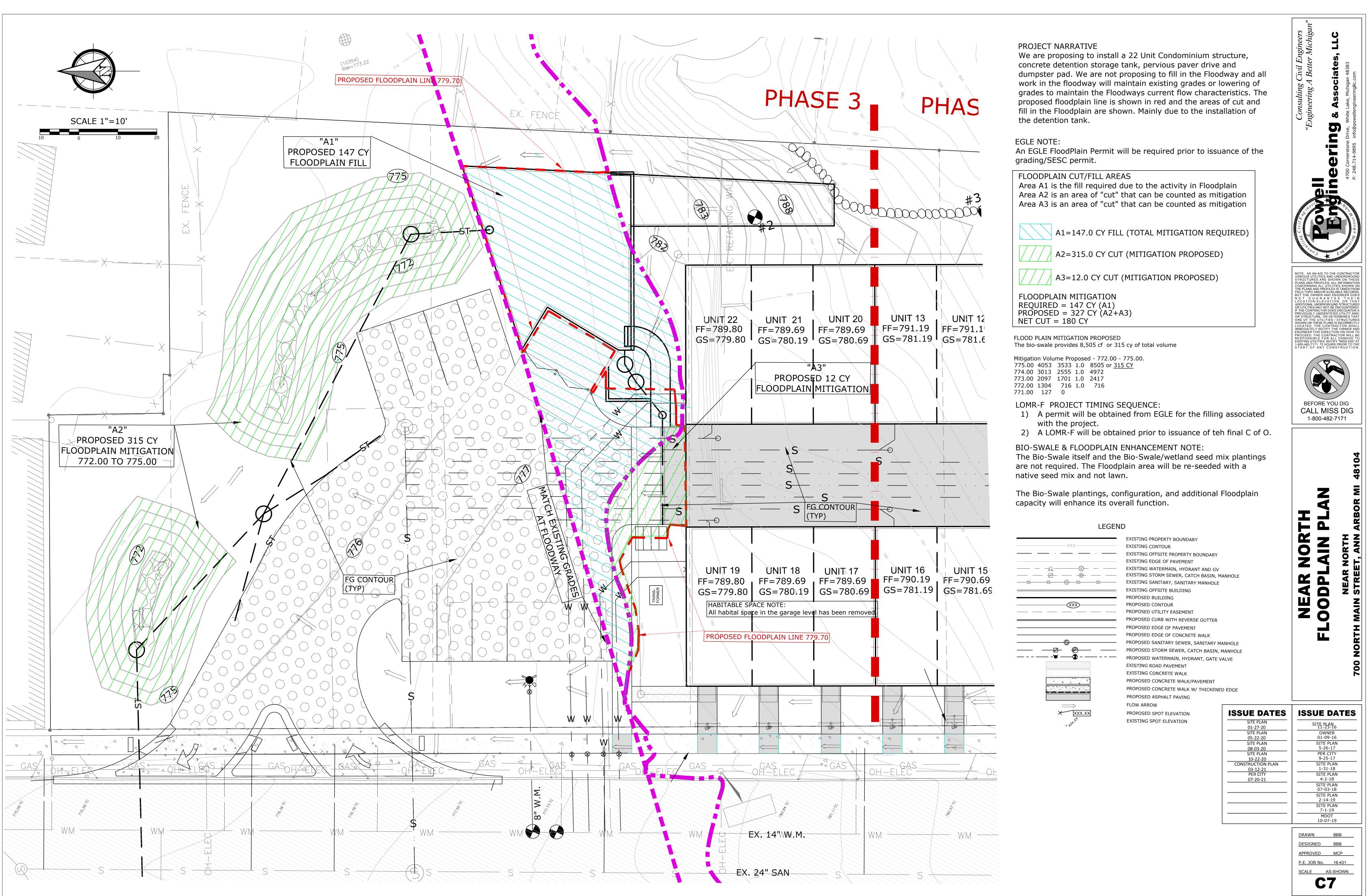






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